

DRAINAGE ANALYSIS

EROSION AND SEDIMENT CONTROL PLAN

PPM TOWNHOMES

Tax Map 3, Lot 37-3 & 37-1 Spaulding Road Fremont, NH 03044

Prepared for:

PPM Fremont Holding, LLC 326 Main Street Fremont, NH 03044



Prepared by:

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October 6, 2021
JBE Project No. 20631

1. EXECUTIVE SUMMARY

The purpose of this project is to construct a 4-unit residential development on Town of Hillsborough Tax Map 3, Lot 37-3, Lot 37-1. The proposed development will contain one 10,000 sq.ft. building with associated parking, drainage, and utilities. Two models were compiled, one for the area in its existing (pre-development) condition, and a second for its proposed (post-development) condition. The analysis was conducted using the USDA SCS TR-20 method within the HydroCAD Stormwater Modeling System environment. A summary of the existing and proposed conditions peak rates of runoff is as follows:

EXECUTIVE SUMMARY TABLE								
Analysis Point	2 Year		10 Year		25 Year		50 Year	
	Pre	Post	Pre	Post	Pre	Post	Pre	Post
Analysis Point #1	2.06	1.40	6.15	5.15	9.99	8.87	13.83	12.98
Analysis Point #2	1.01	1.01	2.10	2.10	3.02	3.02	3.88	3.88
Total	3.07	2.41	8.25	7.25	13.01	11.89	17.71	16.82

The drainage design intent for this site is to maintain the post-development peak flow to the predevelopment peak flow conditions to the extent practicable and to effectively treat stormwater from the development of this project. This has been accomplished through the use of an infiltration basin to maintain the peak discharge and infiltrate stormwater.

In addition, the potential for increased erosion and sedimentation is handled by way of vegetated swales and riprap inlet and outlet protection aprons. The use of Best Management Practices per the NHDES Stormwater Manual have been applied to the design of this drainage system and will be observed during all stages of construction. Existing wetlands and abutting property owners will suffer minimal impact resultant from this development.

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2. DRAINAGE ANALYSIS

2.1 INTRODUCTION

The purpose of this project is to construct a 4-unit residential development on Town of Fremont Tax Map 3, Lot 37-3, Lot 37-1. The proposed development will contain one 10,000 sq.ft. building with associated parking, drainage, and utilities. This project will be serviced by two on-site wells and two on-site septic systems.

2.2 METHODOLOGY

The existing and proposed watersheds were modeled utilizing HydroCad stormwater software, version 9.10. The watersheds were analyzed utilizing the SCS TR-20 methodology for hydrograph development and the TR-55 methodology for Time of Concentration (Tc) determination. The Dynamic-Storage-Indicating method for reach and pond routing was utilized. Type III, 24-hour hydrographs were developed for the 2-year, 10-year, 25-year, and 50-year storm events, corresponding to rainfall events of 3.08", 4.69", 5.96", and 7.14" respectively.

Existing topography and site features were obtained through aerial topography and on-ground topography completed by Jones & Beach Engineers. Existing soil conditions were derived from information obtained from the NRCS Web Soil Survey.

2.3 EXISTING CONDITIONS ANALYSIS

The study area consists of the subject property and upstream contributing area. The study area contains 5.99 acres including offsite contributing areas. The existing site is currently undeveloped with the exception of a gravel driveway and parking area. The existing site features a high point located at the eastern corner of the subject parcel. The site drains to the southwest from this high point. A high point near the center of the subject parcel breaks stormwater and a smaller section of the site abutting Spaulding Road drains to an existing culvert. The Analysis Points are defined below.

The majority of the soils for this site are described as Hydrological Soils "C". A section of soils along Spaulding Rd. are described as Hydrological Soils "B". A smaller section of soils central to the site are described as Hydrological Soils "D". Additionally, portions of the site are covered with exposed ledge.

Two Analysis Points (AP's) were defined for this project. Analysis Points are described as below:

Analysis Point #1 is defined as the existing property line to the south of the subject parcel. Stormwater to this Analysis Point flows overland from the high point at the eastern corner.

Analysis Point #2 is defined as the existing culvert near the existing access driveway. Stormwater bounded by Spaulding Rd. and the high point central to subject parcel drains to this point.

2.4 PROPOSED CONDITIONS ANALYSIS

The purpose of this project is to construct a 4-unit residential development on Town of Fremont Tax Map 3, Lot 37-3, Lot 37-1. The proposed development will contain one 10,000 sq.ft. building with associated parking, drainage, and utilities.

The addition of the proposed impervious paved areas and buildings causes an increase in the curve number (C_n) and a decrease in the time of concentration (T_c) , the net result being a potential increase in peak rates of runoff from the site. To mitigate the potential increase in the peak rate of runoff and to effectively treat the subsequent stormwater runoff the following Best Management Practices (BMP's) have been employed at the Analysis Points as follows:

The access driveway to the site is crowned and ditches line both sides. North of the driveway, the vegetated swale continues to the rear of the proposed building and flows to Infiltration Basin #1. To the west, the vegetated swale follows the perimeter of the parking area and later connects with the swale to the rear of the proposed building. Both swales collect runoff from the proposed building and associated parking and drain to Infiltration Basin #1. Infiltration Basin #1 is released by Outlet Structure #1 and flows to Analysis Point #1. There is a portion of the site that is down grade of Infiltration Basin #1 and this stormwater flows directly to Analysis Point #1.

The portion of the site that drains to Analysis Point #2 remains unchanged.

2.5 CONCLUSION

This proposed site development will have minimal adverse effect on abutting infrastructures or properties by way of stormwater runoff or siltation if properly constructed in accordance with this Drainage Analysis and approved project plan set. The post-construction peak rates of runoff for the site will be lower than the existing conditions for all analyzed storm events. Appropriate steps will be taken to control erosion and sedimentation; these will be accomplished through the construction of a drainage system consisting of site grading, vegetated swales, infiltration basins and riprap outlet protection aprons. The use of Best Management Practices developed by the State of New Hampshire have been utilized in the design of this system and their application will be enforced with regular inspections throughout the construction process.

Respectfully Submitted,

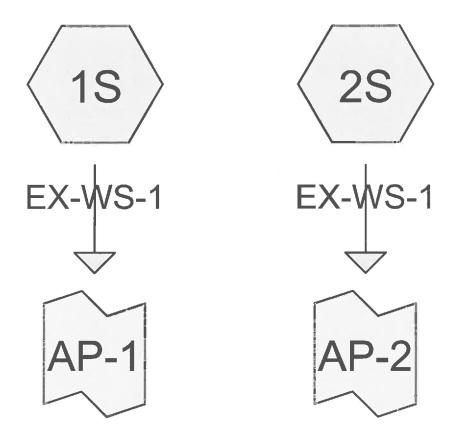
JONES & BEACH ENGINEERS, INC.

Vice-President

2.6 DRAINAGE CALCUALTIONS

PRE-DEVELOPMENT CONDITIONS ANALYSIS

2.6.1	2-Year 24-Hour Summary Analysis
2.6.2	10-Year 24-Hour Complete Analysis
2.6.3	25-Year 24-Hour Summary Analysis
2.6.4	50-Year 24-Hour Summary Analysis













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Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
3.758	61	>75% Grass cover, Good, HSG B (1S)
1.489	74	>75% Grass cover, Good, HSG C (1S, 2S)
0.171	80	>75% Grass cover, Good, HSG D (1S)
0.164	98	Paved roads w/curbs & sewers, HSG B (1S)
0.405	98	Paved roads w/curbs & sewers, HSG C (1S, 2S)
5.988	68	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
3.922	HSG B	18
1.895	HSG C	18, 28
0.171	HSG D	1S
0.000	Other	
5.988		TOTAL AREA

Type III 24-hr 2-Year Rainfall=3.08" Printed 10/5/2021

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: EX-WS-1 Runoff Area=224,843 sf 7.80% Impervious Runoff Depth>0.55"

Flow Length=714' Tc=19.9 min CN=67 Runoff=2.06 cfs 0.238 af

Subcatchment 2S: EX-WS-1 Runoff Area=35,982 sf 20.13% Impervious Runoff Depth>1.15"

Flow Length=383' Tc=10.3 min CN=79 Runoff=1.01 cfs 0.079 af

Link AP-1: Analysis Point 1 Inflow=2.06 cfs 0.238 af

Primary=2.06 cfs 0.238 af

Link AP-2: Analysis Point 2 Inflow=1.01 cfs 0.079 af

Primary=1.01 cfs 0.079 af

Total Runoff Area = 5.988 ac Runoff Volume = 0.317 af Average Runoff Depth = 0.64" 90.49% Pervious = 5.419 ac 9.51% Impervious = 0.569 ac

Type III 24-hr 10-Year Rainfall=4.69"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 15: EX-WS-1 Runoff Area=224,843 sf 7.80% Impervious Runoff Depth>1.44"

Flow Length=714' Tc=19.9 min CN=67 Runoff=6.15 cfs 0.621 af

Subcatchment 2S: EX-WS-1 Runoff Area=35,982 sf 20.13% Impervious Runoff Depth>2.36"

Flow Length=383' Tc=10.3 min CN=79 Runoff=2.10 cfs 0.162 af

Link AP-1: Analysis Point 1 Inflow=6.15 cfs 0.621 af

Primary=6.15 cfs 0.621 af

Link AP-2: Analysis Point 2 Inflow=2.10 cfs 0.162 af

Primary=2.10 cfs 0.162 af

Total Runoff Area = 5.988 ac Runoff Volume = 0.784 af Average Runoff Depth = 1.57" 90.49% Pervious = 5.419 ac 9.51% Impervious = 0.569 ac

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Summary for Subcatchment 1S: EX-WS-1

Runoff = 6.15 cfs @ 12.30 hrs, Volume=

0.621 af, Depth> 1.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.69"

	Α	rea (sf)	CN	Description				
		7,143	98	98 Paved roads w/curbs & sewers, HSG B				
	1	63,716	61	>75% Gras	s cover, Go	ood, HSG B		
		10,405	98	Paved road	s w/curbs 8	& sewers, HSG C		
		36,141	74	>75% Gras	s cover, Go	ood, HSG C		
		7,438	80	>75% Gras	s cover, Go	ood, HSG D		
	2	24,843	67	Weighted A	verage			
	2	207,295		92.20% Per	vious Area			
		17,548		7.80% <mark>I</mark> mpe	ervious Area	a		
	_							
	Tc	Length	Slope			Description		
-	(min)	(feet)	(ft/ft)		(cfs)			
	8.8	100	0.0700	0.19		Sheet Flow,		
						Grass: Dense n= 0.240 P2= 3.08"		
	0.4	42	0.0700	1.85		Shallow Concentrated Flow,		
		400	0.0000	0.00		Short Grass Pasture Kv= 7.0 fps		
	3.2	188	0.0200	0.99		Shallow Concentrated Flow,		
	4.0	400	0 0000	4.04		Short Grass Pasture Kv= 7.0 fps		
	1.8	132	0.0300	1.21		Shallow Concentrated Flow,		
	<i>E</i> 7	252	0.0440	0.72		Short Grass Pasture Kv= 7.0 fps		
	5.7	252	0.0110	0.73		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
-	10.0	71.4	Total			Short Grass Fasture RV-7.0 lps		
	19.9	714	Total					

Summary for Subcatchment 2S: EX-WS-1

Runoff = 2.10 cfs @ 12.15 hrs, Volume=

0.162 af, Depth> 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.69"

 Area (sf)	CN	Description
7,244	98	Paved roads w/curbs & sewers, HSG C
 28,738	74	>75% Grass cover, Good, HSG C
35,982	79	Weighted Average
28,738		79.87% Pervious Area
7,244		20.13% Impervious Area

Type III 24-hr 10-Year Rainfall=4.69"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.4	31	0.0300	1.26		Sheet Flow, ROAD
					Smooth surfaces n= 0.011 P2= 3.08"
6.5	69	0.0700	0.18		Sheet Flow, RD SHOULDER
					Grass: Dense n= 0.240 P2= 3.08"
0.6	78	0.1100	2.32		Shallow Concentrated Flow, GRASS
					Short Grass Pasture Kv= 7.0 fps
2.8	205	0.0300	1.21		Shallow Concentrated Flow, GRASS
					Short Grass Pasture Kv= 7.0 fps
10.3	383	Total			

Summary for Link AP-1: Analysis Point 1

5.162 ac, 7.80% Impervious, Inflow Depth > 1.44" for 10-Year event Inflow Area =

0.621 af Inflow 6.15 cfs @ 12.30 hrs, Volume=

6.15 cfs @ 12.30 hrs, Volume= 0.621 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link AP-2: Analysis Point 2

0.826 ac, 20.13% Impervious, Inflow Depth > 2.36" for 10-Year event Inflow Area =

2.10 cfs @ 12.15 hrs, Volume= 2.10 cfs @ 12.15 hrs, Volume= 0.162 af Inflow

0.162 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Type III 24-hr 25-Year Rainfall=5.96"

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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: EX-WS-1 Runoff Area=224,843 sf 7.80% Impervious Runoff Depth>2.29"

Flow Length=714' Tc=19.9 min CN=67 Runoff=9.99 cfs 0.986 af

Subcatchment 2S: EX-WS-1 Runoff Area=35,982 sf 20.13% Impervious Runoff Depth>3.41"

Flow Length=383' Tc=10.3 min CN=79 Runoff=3.02 cfs 0.235 af

Link AP-1: Analysis Point 1 Inflow=9.99 cfs 0.986 af

Primary=9.99 cfs 0.986 af

Link AP-2: Analysis Point 2 Inflow=3.02 cfs 0.235 af

Primary=3.02 cfs 0.235 af

Total Runoff Area = 5.988 ac Runoff Volume = 1.221 af Average Runoff Depth = 2.45" 90.49% Pervious = 5.419 ac 9.51% Impervious = 0.569 ac

Type III 24-hr 50-Year Rainfall=7.14"

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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: EX-WS-1 Runoff Area=224,843 sf 7.80% Impervious Runoff Depth>3.15"

Flow Length=714' Tc=19.9 min CN=67 Runoff=13.83 cfs 1.356 af

Subcatchment 2S: EX-WS-1 Runoff Area=35,982 sf 20.13% Impervious Runoff Depth>4.42"

Flow Length=383' Tc=10.3 min CN=79 Runoff=3.88 cfs 0.304 af

Link AP-1: Analysis Point 1 Inflow=13.83 cfs 1.356 af

Primary=13.83 cfs 1.356 af

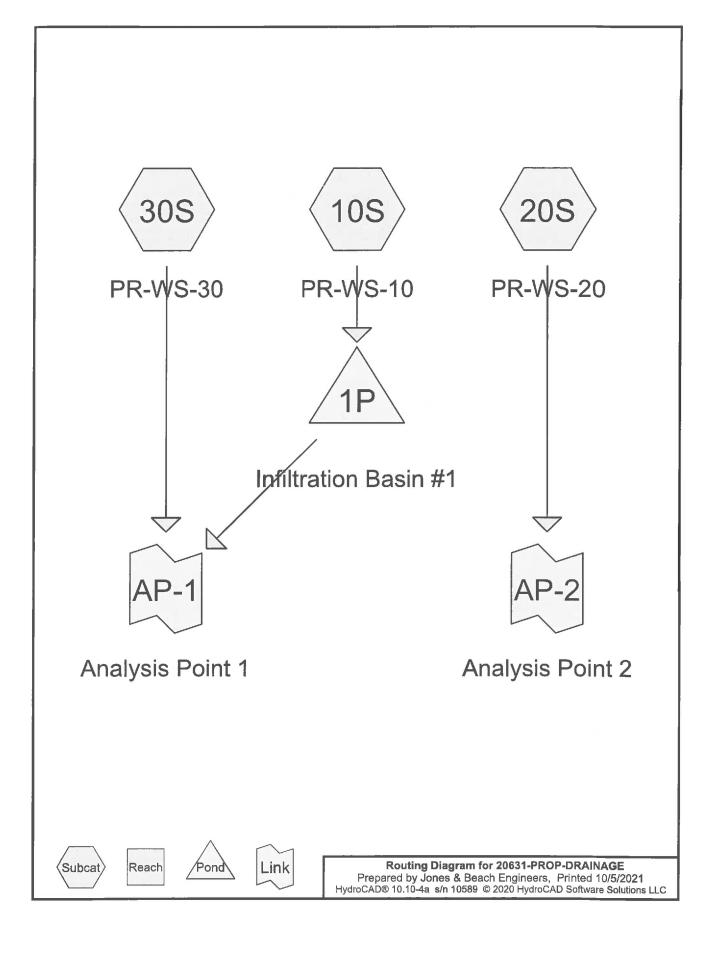
Link AP-2: Analysis Point 2 Inflow=3.88 cfs 0.304 af Primary=3.88 cfs 0.304 af

Total Runoff Area = 5.988 ac Runoff Volume = 1.661 af Average Runoff Depth = 3.33" 90.49% Pervious = 5.419 ac 9.51% Impervious = 0.569 ac

2.7 APPENDIX II

POST-DEVELOPMENT CONDITIONS ANALYSIS

2.7.1	2-Year 24-Hour Summary Analysis
2.7.2	10-Year 24-Hour Complete Analysis
2.7.3	25-Year 24-Hour Summary Analysis
2.7.4	50-Year 24-Hour Summary Analysis



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Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
3.254	61	>75% Grass cover, Good, HSG B (10S, 30S)
1.438	74	>75% Grass cover, Good, HSG C (10S, 20S, 30S)
0.171	80	>75% Grass cover, Good, HSG D (10S)
0.669	98	Paved parking, HSG B (10S)
0.290	98	Paved parking, HSG C (10S, 30S)
0.166	98	Paved roads w/curbs & sewers, HSG C (20S)
5.988	72	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
3.922	HSG B	10S, 30S
1.895	HSG C	10S, 20S, 30S
0.171	HSG D	10S
0.000	Other	
5.988		TOTAL AREA

Type III 24-hr 2-Year Rainfall=3.08"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10S: PR-WS-10 Runoff Area=199,507 sf 20.82% Impervious Runoff Depth>0.77"

Flow Length=914' Tc=13.4 min CN=72 Runoff=3.27 cfs 0.295 af

Subcatchment 20S: PR-WS-20 Runoff Area=35,982 sf 20.13% Impervious Runoff Depth>1.15"

Flow Length=383' Tc=10.3 min CN=79 Runoff=1.01 cfs 0.079 af

Subcatchment 30S: PR-WS-30 Runoff Area=25,336 sf 0.87% Impervious Runoff Depth>0.37"

Flow Length=157' Slope=0.0120 '/' Tc=12.9 min CN=62 Runoff=0.15 cfs 0.018 af

Pond 1P: Infiltration Basin #1 Peak Elev=165.46' Storage=1,855 cf Inflow=3.27 cfs 0.295 af

Discarded=0.79 cfs 0.222 af Primary=1.27 cfs 0.072 af Outflow=2.07 cfs 0.295 af

Link AP-1: Analysis Point 1 Inflow=1.40 cfs 0.090 af

Primary=1.40 cfs 0.090 af

Link AP-2: Analysis Point 2 Inflow=1.01 cfs 0.079 af

Primary=1.01 cfs 0.079 af

Total Runoff Area = 5.988 ac Runoff Volume = 0.392 af Average Runoff Depth = 0.79" 81.21% Pervious = 4.863 ac 18.79% Impervious = 1.125 ac

Type III 24-hr 10-Year Rainfall=4.69"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10S: PR-WS-10

Runoff Area=199,507 sf 20.82% Impervious Runoff Depth>1.80"

Flow Length=914' Tc=13.4 min CN=72 Runoff=8.10 cfs 0.688 af

Subcatchment 20S: PR-WS-20

Runoff Area=35,982 sf 20.13% Impervious Runoff Depth>2.36"

Flow Length=383' Tc=10.3 min CN=79 Runoff=2.10 cfs 0.162 af

Subcatchment 30S: PR-WS-30

Runoff Area=25,336 sf 0.87% Impervious Runoff Depth>1.13"

Flow Length=157' Slope=0.0120 '/' Tc=12.9 min CN=62 Runoff=0.61 cfs 0.055 af

Pond 1P: Infiltration Basin #1

Peak Elev=165.75' Storage=4,537 cf Inflow=8.10 cfs 0.688 af

Discarded=1.07 cfs 0.381 af Primary=4.69 cfs 0.307 af Outflow=5.76 cfs 0.688 af

Link AP-1: Analysis Point 1

Inflow=5.15 cfs 0.362 af

Primary=5.15 cfs 0.362 af

Link AP-2: Analysis Point 2

Inflow=2.10 cfs 0.162 af

Primary=2.10 cfs 0.162 af

Total Runoff Area = 5.988 ac Runoff Volume = 0.905 af Average Runoff Depth = 1.81" 81.21% Pervious = 4.863 ac 18.79% Impervious = 1.125 ac

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Summary for Subcatchment 10S: PR-WS-10

Runoff = 8.10 cfs @ 12.20 hrs, Volume=

0.688 af, Depth> 1.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.69"

A	rea (sf)	CN E	Description				
	29,127	98 F	Paved parking, HSG B				
1	17,837				ood, HSG B		
	12,412			ing, HSG C			
	32,693				ood, HSG C		
	7,438	80 >	75% Gras	s cover, Go	ood, HSG D		
-	99,507	72 V	Veighted A	verage			
	57,968			vious Area			
,	41,539			ervious Are			
	,						
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	'		
6.4	100	0.0600	0.26		Sheet Flow, grass sheet		
					Grass: Short n= 0.150 P2= 3.08"		
0.2	72	0.1250	5.30		Shallow Concentrated Flow, driveway swale		
					Grassed Waterway Kv= 15.0 fps		
0.5	103	0.0600	3.67		Shallow Concentrated Flow, driveway swale		
					Grassed Waterway Kv= 15.0 fps		
5.0	447	0.0100	1.50		Shallow Concentrated Flow, parking lot swale		
					Grassed Waterway Kv= 15.0 fps		
0.7	123	0.0400	3.00		Shallow Concentrated Flow, GRASS SWALE		
					Grassed Waterway Kv= 15.0 fps		
0.6	69	0.0160	1.90		Shallow Concentrated Flow, GRASS SWALE		
					Grassed Waterway Kv= 15.0 fps		
13.4	914	Total					

Summary for Subcatchment 20S: PR-WS-20

Runoff = 2.10 cfs

2.10 cfs @ 12.15 hrs, Volume=

0.162 af, Depth> 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.69"

Area (sf)	CN	Description
 7,244	98	Paved roads w/curbs & sewers, HSG C
 28,738	74	>75% Grass cover, Good, HSG C
35,982	79	Weighted Average
28,738		79.87% Pervious Area
7,244		20.13% Impervious Area

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To (min)		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.4	. 31	0.0300	1.26		Sheet Flow, ROAD
					Smooth surfaces n= 0.011 P2= 3.08"
6.5	69	0.0700	0.18		Sheet Flow, RD SHOULDER
					Grass: Dense n= 0.240 P2= 3.08"
0.6	78	0.1100	2.32		Shallow Concentrated Flow, GRASS
					Short Grass Pasture Kv= 7.0 fps
2.8	205	0.0300	1.21		Shallow Concentrated Flow, GRASS
					Short Grass Pasture Kv= 7.0 fps
10.3	383	Total			

Summary for Subcatchment 30S: PR-WS-30

Runoff = 0.61 cfs @ 12.20 hrs, Volume=

0.055 af, Depth> 1.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.69"

	Α	rea (sf)	CN	Description		
Ī		23,894	61	>75% Gras	s cover, Go	ood, HSG B
		220	98	Paved park	ing, HSG C	
		1,222	74			ood, HSG C
-		25,336	62	Weighted A	verage	
		25,116		99.13% Pe	rvious Area	
		220		0.87% Impe	ervious Area	a
				·		
	Tc	Length	Slop	e Velocity	Capacity	Description
	(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)	·
-	12.3	100	0.012	0 0.14		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.08"
	0.6	57	0.012	0 1.64		Shallow Concentrated Flow,
						Grassed Waterway Kv= 15.0 fps
-	12.9	157	Total			

Summary for Pond 1P: Infiltration Basin #1

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=40)

Inflow Area = 4.580 ac, 20.82% Impervious, Inflow Depth > 1.80" for 10-Year event
Inflow = 8.10 cfs @ 12.20 hrs, Volume= 0.688 af
Outflow = 5.76 cfs @ 12.36 hrs, Volume= 0.688 af, Atten= 29%, Lag= 9.9 min
Discarded = 1.07 cfs @ 12.36 hrs, Volume= 0.381 af
Primary = 4.69 cfs @ 12.36 hrs, Volume= 0.307 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 165.75' @ 12.36 hrs Surf.Area= 9,399 sf Storage= 4,537 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 9.8 min (821.6 - 811.8)

Prepared by Jones & Beach Engineers

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Volume	Inve	rt Avail.	Storage	Storage Description	1		
#1	165.2	5' 9	9,394 cf	Custom Stage Dat	ta (Irregular)Listed	below (Recalc)	
Elevation (fee		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
165.2	25	8,611	511.4	0	0	8,611	
165.7	•	9,393	520.8	4,500	4,500	9,425	
166.2	25	10,189	530.2	4,894	9,394	10,254	
Device	Routing	Inve	ert Outle	et Devices			
#1	Discarded	d 165.2		0 in/hr Exfiltration o			
#2	Primary	166.0	0' 4.0' Head 2.50 Coef	3.00 3.50	EMERGENCY OV 0.60 0.80 1.00 1.2)
#3	Primary	165.2	5' 4.0' l	ong Sharp-Crested	I Vee/Trap Weir Cv	v= 2.62 (C= 3.28)	

Discarded OutFlow Max=1.07 cfs @ 12.36 hrs HW=165.75' (Free Discharge) **1=Exfiltration** (Controls 1.07 cfs)

Primary OutFlow Max=4.67 cfs @ 12.36 hrs HW=165.75' TW=0.00' (Dynamic Tailwater)

2=EMERGENCY OVERFLOW (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 4.67 cfs @ 2.32 fps)

Summary for Link AP-1: Analysis Point 1

Inflow Area = 5.162 ac, 18.57% Impervious, Inflow Depth > 0.84" for 10-Year event

Inflow = 5.15 cfs @ 12.35 hrs, Volume= 0.362 af

Primary = 5.15 cfs @ 12.35 hrs, Volume= 0.362 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link AP-2: Analysis Point 2

Inflow Area = 0.826 ac, 20.13% Impervious, Inflow Depth > 2.36" for 10-Year event

Inflow = 2.10 cfs @ 12.15 hrs, Volume= 0.162 af

Primary = 2.10 cfs @ 12.15 hrs, Volume= 0.162 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Type III 24-hr 25-Year Rainfall=5.96"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10S: PR-WS-10

Runoff Area=199,507 sf 20.82% Impervious Runoff Depth>2.74" Flow Length=914' Tc=13.4 min CN=72 Runoff=12.40 cfs 1.046 af

Subcatchment 20S: PR-WS-20

Runoff Area=35,982 sf 20.13% Impervious Runoff Depth>3.41" Flow Length=383' Tc=10.3 min CN=79 Runoff=3.02 cfs 0.235 af

Subcatchment 30S: PR-WS-30

VS-30 Runoff Area=25,336 sf 0.87% Impervious Runoff Depth>1.88" Flow Length=157' Slope=0.0120 '/' Tc=12.9 min CN=62 Runoff=1.06 cfs 0.091 af

Pond 1P: Infiltration Basin #1

Peak Elev=165.97' Storage=6,613 cf Inflow=12.40 cfs 1.046 af

Discarded=1.28 cfs 0.486 af Primary=8.02 cfs 0.560 af Outflow=9.30 cfs 1.046 af

Link AP-1: Analysis Point 1

Inflow=8.87 cfs 0.651 af Primary=8.87 cfs 0.651 af

Link AP-2: Analysis Point 2

Inflow=3.02 cfs 0.235 af Primary=3.02 cfs 0.235 af

Total Runoff Area = 5.988 ac Runoff Volume = 1.372 af Average Runoff Depth = 2.75" 81.21% Pervious = 4.863 ac 18.79% Impervious = 1.125 ac

Type III 24-hr 50-Year Rainfall=7.14"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10S: PR-WS-10 Runoff Area=199,507 sf 20.82% Impervious Runoff Depth>3.67"

Flow Length=914' Tc=13.4 min CN=72 Runoff=16.60 cfs 1.402 af

Subcatchment 20S: PR-WS-20 Runoff Area=35,982 sf 20.13% Impervious Runoff Depth>4.42"

Flow Length=383' Tc=10.3 min CN=79 Runoff=3.88 cfs 0.304 af

Subcatchment 30S: PR-WS-30 Runoff Area=25,336 sf 0.87% Impervious Runoff Depth>2.67"

Flow Length=157' Slope=0.0120 '/' Tc=12.9 min CN=62 Runoff=1.54 cfs 0.129 af

Pond 1P: Infiltration Basin #1 Peak Elev=166.15' Storage=8,374 cf Inflow=16.60 cfs 1.402 af

Discarded=1.46 cfs 0.572 af Primary=11.75 cfs 0.830 af Outflow=13.21 cfs 1.402 af

Link AP-1: Analysis Point 1 Inflow=12.98 cfs 0.959 af

Primary=12.98 cfs 0.959 af

Link AP-2: Analysis Point 2 Inflow=3.88 cfs 0.304 af Primary=3.88 cfs 0.304 af

Total Runoff Area = 5.988 ac Runoff Volume = 1.835 af Average Runoff Depth = 3.68" 81.21% Pervious = 4.863 ac 18.79% Impervious = 1.125 ac

Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing Yes

State New Hampshire

Location

Longitude 71.137 degrees West **Latitude** 42.991 degrees North

Elevation 0 feet

Date/Time Thu, 03 Jun 2021 10:50:14 **-0400**

Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.50	0.66	0.82	1.04	1yr	0.71	0.99	1.21	1.55	1.99	2.58	2.77	1yr	2.28	2.66	3.08	3.77	4.37	1yr
2yr	0.32	0.50	0.62	0.81	1.02	1,29	2yr	0.88	1.18	1.50	1.91	2.42	3.08	3.42	2yr	2.73	3.29	3.80	4.51	5.15	2yr
5yr	0.38	0.59	0.74	0.99	1.26	1.62	5yr	1.09	1.47	1.89	2.41	3.07	3.91	4.40	5yr	3.46	4.23	4.84	5.75	6.50	5yr
10yr	0.42	0.66	0.84	1.14	1.48	1.92	10yr	1.28	1.73	2.25	2.88	3.68	4.69	5.31	10yr	4.15	5.11	5.82	6.92	7.76	10yr
25yr	0.49	0.79	1.00	1.38	1.83	2.39	25yr	1.58	2.16	2.82	3.64	4.67	5.96	6.83	25yr	5.27	6.56	7.41	8.85	9.82	25yr
50yr	0.56	0.90	1.15	1.60	2.15	2.84	50yr	1.86	2.56	3.37	4.35	5.59	7.14	8.26	50yr	6.32	7.94	8.91	10.66	11.75	50yr
100yr	0.63	1.02	1.31	1.86	2.54	3.38	100yr	2.19	3.03	4.02	5.22	6.71	8.57	10.00	100yr	7.58	9.61	10.71	12.85	14.06	100yr
200yr	0.72	1.17	1.51	2.17	2.99	4.01	200yr	2.58	3.59	4.79	6.24	8.03	10.28	12.10	200yr	9.10	11.64	12.87	15.51	16.83	200yr
500yr	0.85	1.40	1.82	2.65	3.71	5.04	500yr	3.21	4.50	6.04	7.90	10.21	13.09	15.59	500yr	11.58	14.99	16.43	19.90	21.38	500yr

Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.23	0.36	0.44	0.59	0.72	0.88	1yr	0.62	0.86	1.01	1.29	1.56	2.10	2.54	1yr	1.86	2.45	2.83	3.43	3.90	1yr
2yr	0.31	0.48	0.60	0.81	1.00	1.18	2yr	0.86	1.15	1.35	1.79	2.29	2.97	3.27	2yr	2.63	3.14	3.65	4.30	4.91	2yr
5yr	0.36	0.55	0.68	0,93	1.19	1.41	5yr	1.03	1.38	1.60	2.09	2.68	3.55	3.89	5yr	3.14	3.74	4.31	5.38	5.84	5yr
10yr	0.39	0.61	0.75	1.05	1.36	1.61	10yr	1.17	1.57	1.81	2.36	3.01	4.03	4.40	10yr	3.56	4.23	4.87	6.28	6.57	10yr
25yr	0.45	0.69	0.86	1.23	1.62	1.91	25yr	1.39	1.87	2.12	2.74	3.51	4.73	5.18	25yr	4.18	4.98	5.76	7.71	8.52	25yr
50yr	0.50	0.77	0.95	1.37	1.85	2.19	50yr	1.59	2.14	2.38	3.07	3.94	5.32	5.82	50yr	4.71	5.60	6.53	9.01	9.83	50yr
100yr	0.57	0.86	1.07	1.55	2.12	2.50	100yr	1.83	2.45	2.68	3.44	4.42	5.68	6.54	100yr	5.03	6.29	7.43	10.55	11.33	100yr
200yr	0.63	0.95	1.21	1.75	2.44	2.85	200yr	2.10	2.79	3.02	3.86	4.96	6.29	9.13	200yr	5.56	8.78	8.45	12.36	13.08	200yr
500yr	0.74	1.10	1.42	2.06	2.93	3.41	500yr	2.53	3.34	3.53	4.48	5.80	7.15	11.15	500yr	6.33	10.72	10.05	15.26	15.79	500yr

Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.29	0.45	0.54	0.73	0.90	1.08	1yr	0.78	1.05	1.25	1.67	2.11	2.84	3.17	1yr	2.52	3.05	3.50	4.06	4.88	1yr
2yr	0.33	0.51	0.63	0,86	1.05	1,26	2yr	0.91	1.23	1.45	1.91	2.44	3.28	3.64	2yr	2.90	3.50	4.02	4.75	5.49	2yr
5yr	0.41	0.63	0.78	1.07	1.36	1.61	5yr	1.17	1.57	1.85	2.42	3.09	4.29	4.98	5yr	3.80	4.78	5.43	6.15	7.22	5yr
10yr	0.48	0.74	0.92	1.29	1.67	1.96	10yr	1.44	1.92	2.24	2.93	3.71	5.35	6.34	10yr	4.74	6.10	6.84	7.61	9.06	10yr
25yr	0.61	0.93	1.15	1.65	2.17	2.55	25yr	1.87	2.49	2.91	3.76	4.72	7.17	8.79	25yr	6.34	8.45	9.27	10.11	11.15	25yr
50yr	0.72	1.09	1.36	1.96	2.64	3.11	50yr	2.28	3.04	3.53	4.55	5,68	8.95	11.28	50yr	7.92	10.85	11.68	12.53	13.70	50yr
100yr	0.86	1.30	1.63	2.35	3.23	3.79	100yr	2.78	3.70	4.30	5.52	6.84	11.53	14.46	100yr	10.20	13.91	14.71	15.55	16.87	100yr
200yr	1.02	1.54	1.95	2.82	3.93	4.63	200 yr	3.39	4.52	5.25	6.69	8.23	14.51	15.58	200yr	12.84	14.98	18.51	19.30	20.79	200yr
500yr	1.29	1.92	2.47	3.59	5.11	6.02	500yr	4.41	5.89	6.82	8.65	10.53	19.70	21.01	500yr	17.43	20.20	25.09	25.68	27.45	500yr



Project Name:

PPM TOWNHOMES

JBE #:

20631

Town/City:

Fremont, NH

Date:

9/14/2021

Rip Rap Outlet Protection Calculation

Outlet Designation:

RIP RAP #34 (PRETX UNIT #2)

Pipe Size (Do):

36 in.

=

Q10 (cfs):

4.66 cfs

Tailwater Elevation (TW):

0.25 (FT) if TW = 0, assume 3"/0.25'

3 ft

Apron Length (La):

TW<Do

YES

La = 1.8Q/Do^1.5 + 7Do

La =

22.61 ft

TW>Do

No

 $La = 3.0Q/Do^{1.5} + 7Do$

La =

Apron Width (W2)

TW<Do

 $W_2 = 3Do + La$

 $W_2 =$

31.61 ft.

TW>Do

 $W_2 = 3Do + .4La$

 $W_2 =$

ft.

Rip-Rap Diameter (D₅₀):

D₅₀:

 $D_{50} = 0.02Q^{1.3}TW^{*}Do$

 $D_{50} =$

0.20 ft.

2.37 in.

Use 3[™] minimum D₅₀ ==>

D50 =

3 in.

Rip-Rap Thickness (T):

 $T = 2.5*D_{50}$

T =

7.5 in.

Apron Width (W1):

 $W_1 = 3*Do$

 $W_1 =$

9 ft.





Via Email

January 6, 2021 File No. 04.0191032.00

Mr. Barry Gier Jones & Beach Engineers, Inc. 65 Portsmouth Avenue, PO Box 219 Stratham, New Hampshire 03885

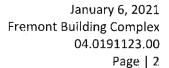
Re: Site-Specific Soil Mapping Report Spaulding Road, Tax Map 3, Lot 37-3 Fremont, New Hampshire

Dear Mr. Gier:

This letter report presents the findings of a Site-Specific Soil Mapping survey conducted on the above-referenced property (Site) by GZA GeoEnvironmental, Inc. (GZA) on December 14, 2020. The subject property is located off of Spaulding Road approximately 355 feet south of the intersection of Spaulding Road and Frost Lane in Fremont, New Hampshire and is identified by the Town as Tax Map 3, Lot 373 (Site). The Site is bordered to the east by Spaulding Road, to the south by commercial property, to the west by undeveloped forest area, and to the north by residential property. The Site is approximately 5.32 acres (see attached plan Concept #1, Spaulding Road, Fremont, NH, dated 9/29/20).

The Site is currently on open and cleared lot consisting predominantly of shrubland. GZA completed wetland delineation on August 13, 2020 and did not observe any jurisdictional wetland systems (see Wetland Delineation Report dated September 9, 2020). We understand that a five-building complex is proposed on the Site and that soil mapping is required to support lot design, road layout, and anticipated Alteration of Terrain permitting for the redevelopment of the Site. This report is subject to the Limitations in Appendix A.

The Site-Specific Soil Mapping survey was conducted by New Hampshire Certified Soil Scientist, James H. Long, (CSS #15) in accordance with the New Hampshire Supplement of the Site-Specific Soil Mapping Standard for New Hampshire and Vermont, Version 4.0, February 2011, published by the Society of Soil Scientists of Northern New England. Soil map units identified on the Site were classified using the New Hampshire State-Wide Numerical Soils Legend, United States Department of Agriculture (USDA) Natural Resource Conservation Service, Issue No. 10, January 2011. The Site-Specific Standards are based on a universally recognized taxonomic system of soil classification and are supported by national soil mapping standards established by the USDA National Cooperative Soil Survey. This map has





been prepared to be consistent with soil mapping requirements outlined in RSA 485 A:17 and New Hampshire Department of Environmental Services (DES) Env-Wq 1500, Alteration of Terrain (AoT) rules. Site-Specific Soil Mapping boundaries were sketched by GZA on the plan titled "Concept #1," dated September 29, 2020 (See Appendix B).

This report provides soil information including soil drainage classification, physical characteristics, and depth to bedrock (if encountered). Soil characteristics on the property were assessed through the evaluation test pits conducted throughout the property on December 14, 2020 (see **Appendix C**). Tile spade and hand auger probe observations were also evaluated to fill in gaps in areas that were not test pitted. Slope phases were measured by the topography shown on the Plan, which depicts the area of the Site mapped at a scale of 1-inch equals 30 feet and provides topographic information at a contour interval of 2 feet.

Soil parent materials encountered consist of disturbed cut and fill material, excavated material, and man-made material. The disturbed soil units identified during the soil survey are briefly described below. Soil characteristics for each of these units comply with the Disturbed Soil Mapping Unit Supplement for DES AoT (Appendix D). The table below provides a description of each Soil Map Unit identified on the Site during the field soil survey:

MAP UNIT NO.	SOIL TAXONOMIC NAME	DESCRIPTION
1998/haade	Fili Material 3-8% slope	This map unit represents areas that have been filled with large boulders, broken ledge, as well as fill material. These soils have an undetermined drainage class, no natural soil within 60 inches, no restrictive layers, an undetermined KSat value, and an undetermined hydrologic soil group. Slopes range from 3-8%.
199C/haade	Fill Material 8-15% slope	This map unit represents areas that have been filled with large boulders, broken ledge, as well as fill material. These soils have an undetermined drainage class, no natural soil within 60 inches, no restrictive layers, an undetermined KSat value, and an undetermined hydrologic soil group. Slopes range from 8-15%.
199E/haade	Fill Material 25-50% slope	This map unit represents areas that have been filled with large boulders, broken ledge, as well as fill material. These soils have an undetermined drainage class, no natural soil within 60 inches, no restrictive layers, an undetermined KSat value, and an undetermined hydrologic soil group. Slopes range from 25-50%.
299A/daabb	Udorthents, smoothed 0-3% slope	This map unit represents an area that has been filled. These soils are moderately well drained, no natural soil within 60 inches, no restrictive layers, has a moderate KSat value and are in Hydrologic Soil Group B. Slopes range from 0-3%.
299B/daabb	Udorthents, smoothed 3-8% slope	This map unit represents an area that has been filled. These soils are moderately well drained, no natural soil within 60 inches, no restrictive layers, has a moderate KSat value and are in Hydrologic Soil Group B. Slopes range from 3-8%.
299B/dbabb	Udorthents, smoothed 3-8% slope	This map unit represents an area that has been filled. These soils are moderately well drained, consist of glacial fluvial deposits, has no restrictive layers, has a moderate KSat value and are in Hydrologic Soil Group B. Slopes range from 3-8%.
2998/eafcd	Udorthents, smoothed 3-8% slope	This map unit represents an area that has been filled. These soils are somewhat poorly drained, no natural soil within 60 inches, has variable depths to bedrock, has a low KSat value, and are in Hydrologic Soil Group D. Slopes range from 3-8%.
299C/dbaab	Udorthents, smoothed 8-15% slope	This map unit represents an area that has been filled. These soils are moderately well drained, consist of glacial fluvial deposits, has no restrictive layers, has a high KSat value and are in Hydrologic Soil Group B. Slopes range from 8-15%.
350A/dbaab	Udipsamments 0-3% slope	This map unit represents an area that has been excavated. These soils are moderately well drained, consist of glacial fluvial deposits, has no restrictive layers, has a high KSat value and are in Hydrologic Soil Group B. Slopes range from 0-3%.
350C/dbaab	Udipsamments 8-15% slope	This map unit represents an area that has been excavated. These soils are moderately well drained, consist of glacial fluvial deposits, has no restrictive layers, has a high KSat value and are in Hydrologic Soil Group B. Slopes range from 8-15%.



MAP UNIT NO.	SOIL TAXONOMIC NAME	DESCRIPTION
500B/dcccc	Udorthents, loamy 3-8% slope	This map unit represents an area that has been excavated. These soils are moderately well drained, consist of glacial till material and a mineral restrictive layer within 40 inches, has a low KSat value and are in Hydrologic Soil Group C. Slopes range from 3-8%.
500C/dcccc	Udorthents, loamy 8-15% slope	This map unit represents an area that has been excavated. These soils are moderately well drained, consist of glacial till material and a mineral restrictive layer within 40 inches, has a low KSat value and are in Hydrologic Soil Group C. Slopes range from 8-15%.
500D/dcccc	Udorthents, loamy 15-25% slope	This map unit represents an area that has been excavated. These soils are moderately well drained, consist of glacial till material and a mineral restrictive layer within 40 inches, has a low KSat value and are in Hydrologic Soil Group C. Slopes range from 15-25%.
550C/hcdde Rk	Udorthents, Bedrock 8-15%	This map unit represents an area that has been excavated. These soils have an undetermined drainage class, consist of glacial till material, bedrock within the first 20 inches, an undetermined KSat value, and has an undetermined hydrologic soil group. Slopes range from 8-15%.
550D/hcdde Rk	Udorthents, Bedrock 15-25%	This map unit represents an area that has been excavated. These soils have an undetermined drainage class, consist of glacial till material, bedrock within the first 20 inches, an undetermined KSat value, and has an undetermined hydrologic soil group. Slopes range from 15-25%.
550E/hcdde Rk	Udorthents, Bedrock 25-50%	This map unit represents an area that has been excavated. These soils have an undetermined drainage class, consist of glacial till material, bedrock within the first 20 inches, an undetermined KSat value, and has an undetermined hydrologic soil group. Slopes range from 25-50%.
550F/hcdde Rk	Udorthents, Bedrock >50%	This map unit represents an area that has been excavated. These soils have an undetermined drainage class, consist of glacial till material, bedrock within the first 20 inches, an undetermined KSat value, and has an undetermined hydrologic soil group. Slopes range is greater than 50%.

GZA trusts this letter report meets your current planning needs. Once you have reviewed the report, please let us know if you require additional information. Please do not hesitate to contact Mr. James Long at 603-235-6993 if you have any questions.

Very truly yours,

GZA GEOENVIRONMENTAL, INC.

Lindsey White, Apprentice Soil Scientist

Assistant Project Manager

James H. Long, CPSS, CSS, CWS Senior Technical Specialist

Associate Principal

Attachments: Appendix A – Natural Resource Limitations

Appendix B – Site-Specific Soil Map

Appendix C - Test Pit Logs

Appendix D – Disturbed Soil Mapping Unit Supplement for DES AoT





NATURAL RESOURCE SURVEY AND ASSESSMENT LIMITATIONS

Use of Report

1. GZA GeoEnvironmental, Inc. (GZA) has prepared this report on behalf of, and for the exclusive use of Jones and Beach Engineers, Inc. ("Client") for the stated purpose(s) and location(s) identified in the report. Use of this report, in whole or in part, at other locations, or for other purposes, may lead to inappropriate conclusions; and we do not accept any responsibility for the consequences of such use(s). Further, reliance by any party not identified in the agreement, for any use, without our prior written permission, shall be at that party's risk, and without any liability to GZA.

Standard of Care

- 2. GZA's findings and conclusions are based on the work conducted as part of the Scope of Services set forth in the Report and/or proposal, and reflect our professional judgment. These findings and conclusions must be considered not as scientific or engineering certainties, but rather as our professional opinions concerning the data gathered and observations made during the course of our work. Conditions other than described in this report may be found at the subject location(s).
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- 4. Natural resource characteristics are inherently variable. Biological community composition and diversity can be affected by seasonal, annual or anthropogenic influences. In addition, soil conditions are reflective of subsurface geologic materials, the composition and distribution of which vary spatially.
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December 2019 PAGE 1

Compliance with Regulations and Codes

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New Information

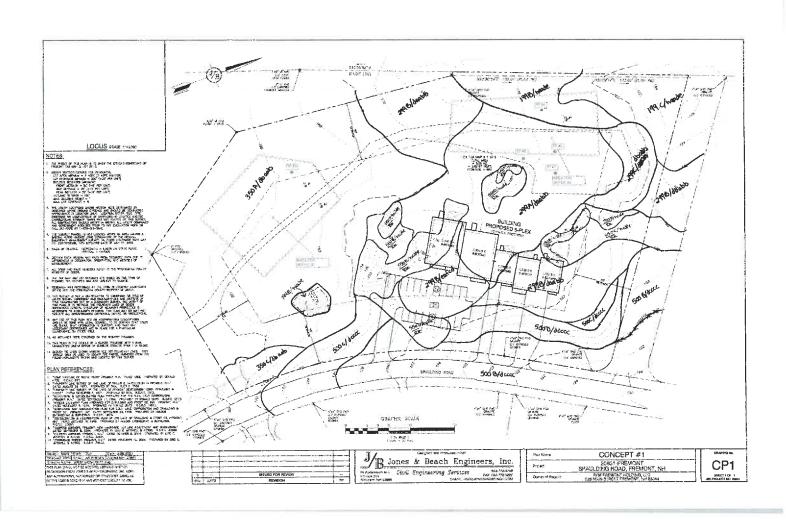
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December 2019 PAGE 2









File No. 04.0191123.00

Evaluated by:	James H. Long, CSS	Designer:	988	Witnessed by:	Larry Miner	Date:	12/14/20
Test Pit No.	1	NOTES:					
Depth (inches)	Description						
^0-4	10YR3/3 Dark brown, ve	ry fine sand	y loam, grar	nular, friable (fill))		
^4-16	2.5Y4/4 Olive brown, loamy sand, granular, friable (fill)						
^16-32	10YR4/4 Dark yellowish i	orown, grav	elly loamy s	and, single grain	, loose (fill)		
32-88	2.5Y5/3 Light olive brown	n, course gr	avelly loam	y sand, single gra	in, loose		
	2.5Y6/1 Gray and 7.5YR4	/6 Strong b	rown redox	imorphic feature	es		
Estimated Seas	onal High Water Table @	32	inches	Observed ¹	Water Table @	76	inches
	Restrictive @	none	inches		Roots @	12	inches
	Refusal @ = Percolation Rate	none 2	inches	Inch @ 26			
Test Pit No.	2	NOTES: I	Possible bed	drock at 32 inch	es at east end of	test pit.	121
Test Pit No. Depth (inches)		NOTES: I	Possible bed	drock at 32 inch	es at east end of	test pit.	
Depth (inches)						test pit.	
Depth (inches)	Description	y fine sand	/ loam, grar	nular, frìable (fill)		test pit.	
Depth (inches) ^0-4 ^4-16	Description 10YR3/3 Dark brown, ver	ry fine sand my sand, gr	y loam, grar anular, friab	nular, frìable (fill) ble (fill)	1	test pit.	
Depth (inches)	Description 10YR3/3 Dark brown, ver 2.5Y4/4 Olive brown, loa	ry fine sand my sand, gr prown, grav	y loam, grar anular, frial elly loamy s	nular, friable (fill) ble (fill) and, single grain	, loose (fill)	test pit.	
Depth (inches) ^0-4 ^4-16 ^16-32	Description 10YR3/3 Dark brown, ver 2.5Y4/4 Olive brown, loa 10YR4/4 Dark yellowish I	ry fine sand my sand, gr prown, grav n, course gra	y loam, gran anular, friab elly loamy s avelly loam	nular, friable (fill) ble (fill) and, single grain y sand, single gra	, loose (fill) nin, loose	test pit.	
^0-4 ^4-16 ^16-32 32-76	Description 10YR3/3 Dark brown, ver 2.5Y4/4 Olive brown, loa 10YR4/4 Dark yellowish I 2.5Y5/3 Light olive brown	ry fine sand my sand, gr prown, grav n, course gra	y loam, gran anular, friab elly loamy s avelly loam	nular, friable (fill) ple (fill) and, single grain y sand, single gra morphic feature	, loose (fill) nin, loose	test pit.	inches
^0-4 ^4-16 ^16-32 32-76	Description 10YR3/3 Dark brown, ver 2.5Y4/4 Olive brown, loa 10YR4/4 Dark yellowish I 2.5Y5/3 Light olive brown 2.5Y6/1 Gray and 7.5YR4	ry fine sand my sand, gr orown, grav n, course gra /6 Strong b	y loam, gran anular, friab elly loamy s avelly loamy rown redoxi	nular, friable (fill) ple (fill) and, single grain y sand, single gra morphic feature	, loose (fill) iin, loose is		
^0-4 ^4-16 ^16-32 32-76	Description 10YR3/3 Dark brown, ver 2.5Y4/4 Olive brown, loa 10YR4/4 Dark yellowish I 2.5Y5/3 Light olive brown 2.5Y6/1 Gray and 7.5YR4 onal High Water Table @	ry fine sand my sand, gr prown, grav n, course gra /6 Strong bi	y loam, gran anular, friab elly loamy s avelly loamy rown redoxi inches inches inches	nular, friable (fill) ple (fill) and, single grain y sand, single gra morphic feature	, loose (fill) in, loose s Water Table @	76	inches



File No. 04.0191123.00

Evaluated by:	James H. Long, CSS	Designer:	988	Witnessed by:	Larry Miner	Date:	12/14/20
Test Pit No.	3	NOTES:					
Depth (inches)	Description						
^0-4	10YR3/3 Dark brown, ve	ry fine sandy	y loam, gra	nular, friable (fill)		
^4-20	2.5Y4/4 Olive brown, gra	velly loamy	sand, grar	ular, friable (fill)			
^20-42	2.5Y5/4 Light olive brow	n, course gra	avelly loan	ny sand, single gra	ain, loose (fill)		
42-46	2.5Y2.5/1 Black, loamy v	ery fine sand	d, granular	, friable			
46-54	10YR4/4 Dark yellowish l	brown, fine :	sandy loan	n, granular, friabl	e		
54-60	-60 10YR5/4 Yellowish brown, fine sandy loam, granular, friable						
60-72	2.5Y6/3 Light yellowish b	rown, grave	elly course	sand, single grair	ı, loose		
	2.5Y6/1 Gray and 7.5YR4	/6 Strong bi	own redo	kimorphic feature	25		
Estimated Seas	onal High Water Table @	60	inches	Observed	Water Table @	none	inches
	Restrictive @	none	inches		Roots @	54	inches
	Refusal @	none	inches	/			
	Percolation Rate =	2	Minutes	/ Inch @ 42			
Test Pit No.	4 (Failed)	NOTES: U	Jnsuitable	due to large bur	ied boulders wit	h fill on t	op.
Depth (inches)	Description						
5		NI/A	inahas	Observed	Matar Table @	N/A	inches
Estimated Seas	onal High Water Table @ @ Restrictive	N/A N/A	inches inches	Observed	Water Table @ Roots @	N/A N/A	inches inches
	Refusal @	N/A	inches			,	inches
	Percolation Rate =	N/A		/ Inch @			



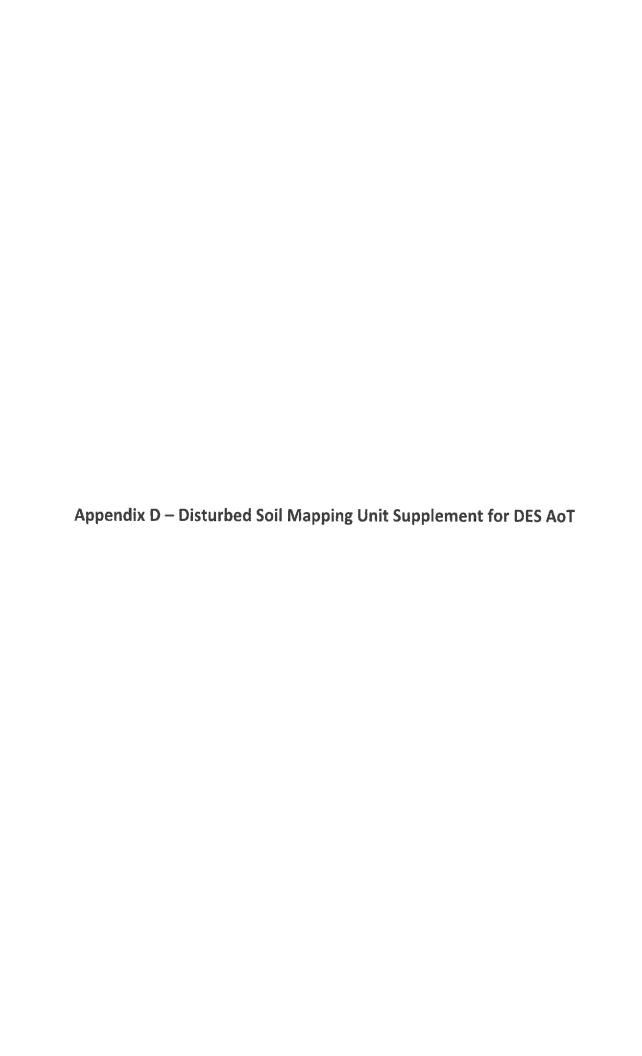
File No. 04.0191123.00

Evaluated by:	James H. Long, CSS	Designer:	988	Witnessed by:	Larry Miner	Date:	12/14/20
Test Pit No.	5	NOTES:					
Depth (inches)	Description						
^0-2	10YR3/3 Dark brown, ver	ry fine sand	y loam, gr	anular, friable (fill)		
^2-26	2.5Y5/3 Light olive brown	n, very fine :	sand, gran	ular, friable (fill)			
^26-60	2.5Y5/3 Light olive brown	n, very fine	sand, gran	ular, friable (fill)			
	2.5Y6/2 Light brownish g	ray and 7.5	YR4/6 Stro	ong brown redoxir	morphic features		
Estimated Seas	onal High Water Table @	26	inches	Observed	Water Table @	52	inches
	Restrictive @	none	inches		Roots @	20	inches
	Refusal @	60	inches				
	Percolation Rate =	10	winutes	/ Inch @ 24			
Test Pit No.	6	NOTES:					
Depth (inches)	Description						
^0-2	10YR3/3 Dark brown, ver	y fine sandy	y loam, gra	anular, friable (fill)		
^2-30	2.5Y5/3 Light olive brown	n, very fine :	sand, gran	ular, friable (fill)			
^30-66	2.5Y5/3 Light olive brown	n, very fine :	sand, gran	ular, friable (fill)			
	2.5Y6/2 Light brownish g	ray and 7.5	YR4/6 Stro	ong brown redoxir	morphic features		
Estimated Seas	onal High Water Table @	36	inches	Observed	Water Table @	60	inches
	Restrictive @	none	inches		Roots @	16	inches
	Refusal @	66	inches				
	Percolation Rate =	8	Minutes	/ Inch @ 24			



File No. 04.0191123.00

Witnessed by: Larry Miner Date: 12/14/20 Evaluated by: James H. Long, CSS Designer: 988 Test Pit No. 7 **NOTES:** Depth (inches) Description 2.5Y5/3 Light olive brown, gravelly loamy sand, granular, friable (fill) ^0-32 2.5Y6/3 Light yellowish brown, loamy sand, massive, firm 32-46 2.5Y6/2 Light brownish gray and 7.5YR4/6 Strong brown redoximorphic features inches Observed Water Table @ inches Estimated Seasonal High Water Table @ 32 26 inches Roots @ 12 inches Restrictive @ 32 Refusal @ 46 inches Minutes / Inch @ 24 Percolation Rate = 12



Supplemental Symbols

The five components of the Disturbed Soil Mapping Unit Supplement are as follows:

Symbol 1: Drainage Class

a-Excessively Drained

b-Somewhat Excessively Drained

c-Well Drained

d-Moderately Well Drained

e-Somewhat Poorly Drained

f-Poorly Drained

g-Very Poorly Drained

h-Not Determined

Symbol 2: Parent Material (of naturally formed soil only, if present)

a-No natural soil within 60"

b-Glaciofluvial Deposits (outwash/terraces of sand or sand and gravel)

c-Glacial Till Material (active ice)

d-Glaciolacustrine very fine sand and silt deposits (glacial lakes)

e-Loamy/sandy over Silt/Clay deposits

f-Marine Silt and Clay deposits (ocean waters)

g-Alluvial Deposits (floodplains)

h-Organic Materials-Fresh water Bogs, etc

i- Organic Materials-Tidal Marsh

Symbol 3: Restrictive/Impervious Layers

a-None

b-Bouldery surface with more than 15% of the surface covered with boulders

c-Mineral restrictive layer(s) are present in the soil profile less than 40 inches below the soil surface such as hard pan, platy structure or clayey texture with consistence of at least firm (i.e. more than 20 newtons). For other examples of soil characteristics that qualify for restrictive layers, see "Soil Manual for Site evaluations in NH" 2nd Ed., (page 3-17, figure 3-14)

d-Bedrock in the soil profile; 0-20 inches

e-Bedrock in the soil profile; 20-60 inches

f-Areas where depth to bedrock is so variable that a single soil type cannot be applied, will be mapped as a complex of soil types

g-Subject to Flooding

h-Man-made impervious surface including pavement, concrete, or built-up surfaces (i.e. buildings) with no morphological restrictive layer within control section

Symbol 4: Estimated Ksat* (most limiting layer excluding symbol 3h above).

a- High.

b-Moderate

c-Low

d-Not determined

*See "Guidelines for Ksat Class Placement" in Chapter 3 of the Soil Survey Manual, USDA

Symbol 5: Hydrologic Soil Group*

a-Group A

b-Group B

c-Group C

d-Group D

e-Not determined

^{*}excluding man-made surface impervious/restrictive layers





VIA EMAIL

September 9, 2020 File No. 04.0191123.00

Mr. Barry Gier, P.E. Jones & Beach Engineers, Inc. P.O. Box 219 Stratham, New Hampshire 03885

Re: Wetland Delineation Report
Jones & Beach Site Development
Spaulding Road, Tax Map 3, Lot 37-3
Fremont, New Hampshire

Dear Mr. Gier:

GZA GeoEnvironmental, Inc. (GZA) is pleased to provide this letter report detailing the completion of wetland delineation field work at the parcel identified as Tax Map 3, Lot 37-3, located off Spaulding Road in Fremont, New Hampshire (i.e. Site). The Site is approximately 5.32 acres and is located approximately 355 feet south of the intersection of Spaulding Road and Frost Lane in Fremont, New Hampshire. It is bordered to the east by Spaulding Road, to the south by a commercial property, to the west by undeveloped forest, and to the north by residential property. GZA understands the Site is a prior sand and gravel operation where sand/gravel material was removed and resulted in exposed ledge outcrops and high-water tables. The delineation field work was performed by Mr. James Long, State of New Hampshire Certified Wetland Scientist (#007) and Certified Soil Scientist (#015), on August 13, 2020. This report is subject to the attached Limitations (see Appendix A).

The purpose of the work completed was to evaluate and locate the boundaries of wetlands on the Site. GZA understands that the data from the wetland delineation will be used in permit applications for the construction of the proposed Spaulding Road, Fremont Site Development.

The wetland delineation was conducted in accordance with the 1987 Corps of Engineers Corps of Engineers Wetlands Delineation Manual¹, using the Routine Determination Method; in conjunction with the Regional Supplement² to the Corps of Engineers Wetland Delineation Manual, the National Plant List: 2016 wetlands rakings³, Field Indicators of

¹ U.S. Army Corps of Engineers, Environmental Laboratory. 1987. *Corps of Engineers Wetlands Delineation Manual*, Technical Report Y-87-1, U.S. Army Engineer Waterways Experiment Station, Vicksburg, Mississippi.

² U.S. Army Corps of Engineers, 2012. *Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Northcentral and Northeast Region* (Version 2.0), ed. J.S. Wakeley, R.W. Lichvar, C.V. Noble and J.F. Berkowitz. ERDC/ELTR-12-1. Vicksburg, Mississippi; U.S. Army Engineer Research and Development Center.

³ Lichvar, R.W., D.L. Banks, W.N. Kirchner, and N.C. Melvin. 2016. *The National Plant List: 2016 wetland rakings*. Phytoneuron 2016-30: 1-17.



September 9, 2020 File No. 04.0191123.00 Spaulding Road Site Development, Fremont, NH |Page | 2

Hydric Soils in the United States Version 8.1,4 and Field Indicators for Identifying Hydric Soils in New England.⁵ Observed wetlands were classified in accordance with the Classification of Wetlands and Deepwater Habitats of the United States.⁶

The presence of potential vernal pools was evaluated in accordance with *Identification and Documentation of Vernal Pools in New Hampshire*, Third Edition, 2016, New Hampshire Fish and Game Department, Nongame and Endangered Wildlife Program. Vernal pool areas exist as confined basins and exhibit vernal pool criteria as outlined in the New Hampshire Code of Administrative Rules, Env-Wt 101.75, 101.86, and 101.106.

The Site consists predominantly of early successional habitat, with exposed ledge outcrops (see Photo Log, **Appendix B**). No wetlands or potential vernal pools were observed on Site during the wetland delineation. Vegetation in excavated areas throughout the site included birdsfoot trefoil (*Lotus corniculatus*), goldenrod (*Solidago spp.*), purple loosestrife (*Lythrum salicaria*), tall scouring-rush (*Equisetum hyemale*), blunt broom sedge (*Carex tribuloides*), umbrella-sedge (*Cyperus strigosus*), common rush (*Juncus effusus*), Canada rush (*Juncus canadensis*), boneset thoroughwort (*Eupatorium perfoliatum*), hollow joe-pye weed (*Eutrochium fistulosum*), sweet fern (*Comptonia peregrina*), Queen Anne's lace (*Daucus carota*), broad-leaved cattail (*Typha latifolia*), willow (*Salix spp.*), gray birch (*Betula populifolia*), and autumn olive (*Elaeagnus umbellata*).

GZA identified one previous gravel pit excavation area with hydrophytic vegetation in the northeastern portion of the Site. This excavation area was reviewed for hydrology, hydric soil criteria and hydrophytic vegetation (see Wetland Determination data sheet, **Appendix C**). Dominant plants observed in the excavation area include willow, birdsfoot trefoil and tail scouring-rush. GZA concluded that although hydrophytic vegetation and a high groundwater table are present, the areas observed do not meet current hydric soils criteria, and therefore the assessed area is not a jurisdictional wetland. The hydrophytic plant communities observed on Site are what GZA believes to be the result of prior excavation of sand and gravel to groundwater table. This evidence can be seen in historic aerial photographs (see **Appendix D**). The gravel pit operation is was not visible on the 2003 aerial, but is clearly present on the 2008 aerial. After discussions during the August 13, 2020 field work with the landowner, Tom Nisbet, GZA understands Mr. Nisbet was required by the town to add fill to low-lying areas as part of gravel pit reclamation on the site. Further, GZA notes the Site was delineated as part of the October 8, 2004 Site Plan labeled Coopers Corner, Tax Map 3, Lots 37 & 37-1, prepared by Eric C. Mitchell & Associates Inc (highlighted in yellow on figure). Wetlands were identified near Route 107 but not on the Site, which supports the assumption that hydrology was not present until excavation for gravel pit operation (see **Appendix E**).

Please feel free to contact the undersigned if you have any questions regarding this Wetland Delineation Report.

United States Department of Agriculture, Natural Resource Conservation Service, 2018. Field Indicators of Hydric Soils in the United States, Version 8.2. L.M. Vasilas, G.W. Hurt, and J.F. Berkowitz (eds.). USDA, NRCS, in cooperation with the National Technical Committee for Hydric Soils.

⁵ New England Hydric Soils Technical Committee. 2017 Version 4. Field Indicators for Identifying Hydric Soils in New England, New England Interstate Water Poliution Control Commission, Lowell, Massachusetts.

⁶ Federal Geographic Data Committee. 2013. *Classification of Wetlands and Deepwater Habitats of the United States*. FGDC-STD-004-2013. Federal Geographic Data Committee and U.S. Fish and Wildlife Service.



September 9, 2020 File No. 04.0191123.00 Spaulding Road Site Development, Fremont, NH Page | 3

Debruh Il- Java a.

Deborah M. Zarta Gier, CNRP

Consultant / Reviewer

James Long, CWS, CSS

Very truly yours,

GZA GEOENVIRONMENTAL, INC.

Lindsey White

Assistant Project Manager

Tracy L. Tarr, CWS, CESSWI

Associate Principal

LEW/TLT/DMZ/JHL

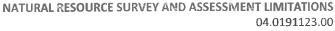
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Appendix B – Photo Log

Appendix C – Wetland Determination Data Form

Appendix D – Historic Aerial Overlays Appendix E – Coopers Corner Site Plan









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GZN)

NATURAL RESOURCE SURVEY AND ASSESSMENT LIMITATIONS

04.0191123.00 Page | 2 August 2020

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PHOTO LOG Spaulding Road, Fremont Site Development Fremont, New Hampshire

Photos Taken: August 13, 2020



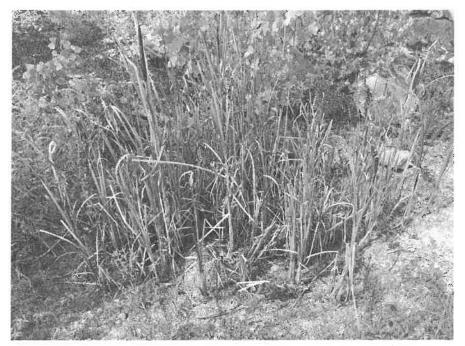
Photograph No. 1: Looking westerly into the Site from Spaulding Road.



Photograph No. 2: Looking northwesterly at an existing access road from Spaulding Road in the southwestern portion of the Site.

PHOTO LOG Spaulding Road, Fremont Site Development Fremont, New Hampshire

Photos Taken: August 13, 2020



Photograph No. 3: Looking into a low-lying area in the center of the site containing broad-leaved cattail, purple loosestrife, and gray birch. This area has been previously excavated and hydric soils were not observed.



Photograph No. 4: Looking easterly at dense vegetation near the northern portion of the Site.

PHOTO LOG Spaulding Road, Fremont Site Development Fremont, New Hampshire

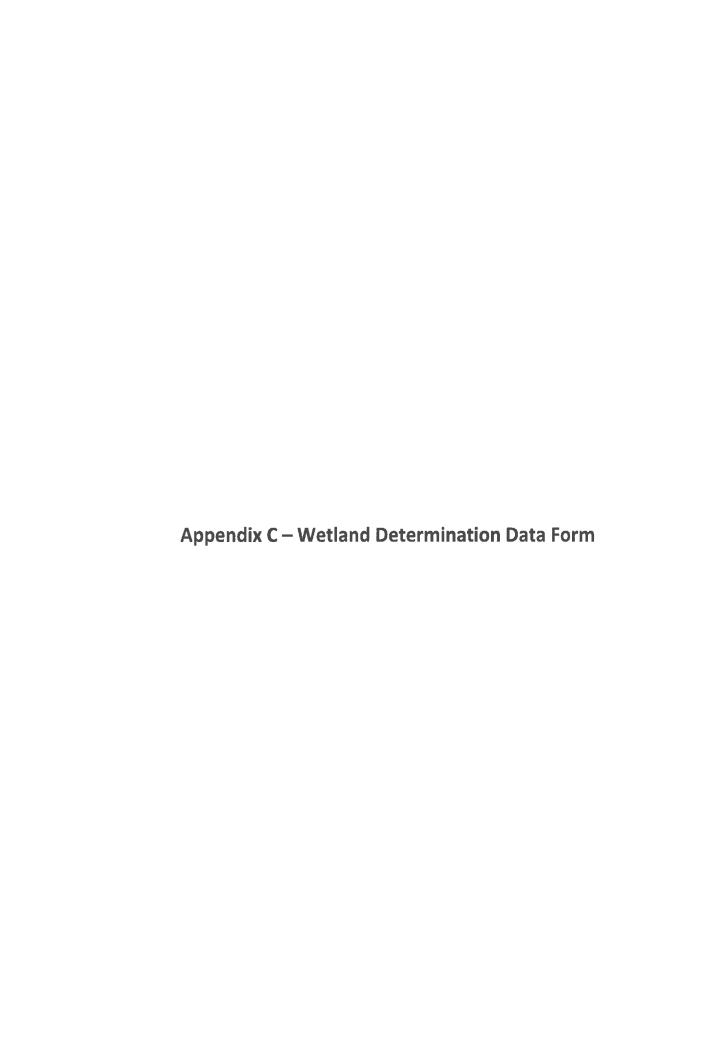
Photos Taken: August 13, 2020



Photograph No. 5: Looking northerly toward the center of the Site at exposed ledge and existing gravel piles.



Photograph No. 6: looking northwesterly into dense vegetation from Spaulding Road.



WETLAND DETERMINATION DATA FORM – Northcentral and Northeast Region

Project/Site: Spaulding Road Tax Map 3, Lots 37-3 City/Col	unty: Fremont Sampling Date: August 13, 2020
Applicant/Owner: PPM Fremont Holdings, LLC, Attn: Thomas Nisbet	State: NH Sampling Point: 1A
Investigator(s): GZA GeoEnvironmental, Inc. Section	, Township, Range:
Landform (hillstope, terrace, etc.): Local relief	f (concave, convex, none): Stope (%): 3-15%
Subregion (LRR or MLRA): Lat: 42.991323 N	Long: -71.137556 W Datum:
Soil Map Unit Name: 799 Urban land-Canton complex	NWI classification:
Are climatic / hydrologic conditions on the site typical for this time of year? Yes	s X No (If no, explain in Remarks.)
Are Vegetation Soil or Hydrology significantly disturbed	
Are Vegetation Soil or Hydrology naturally problemati	· ·
SUMMARY OF FINDINGS – Attach site map showing same	oling point locations, transects, important features, etc.
Hydrophytic Vegetation Present? Yes X No	Is the Sampled Area
Trydrophytic Vegetation resent:	within a Wetland? Yes No X
	If yes, optional Wetland Site ID:
Remarks: (Explain alternative procedures here or in a separate report.) Community type: Select from list	
The site is an old sand and gravel operation with ex excavated near or at groundwater table, allowing for	posed ledge outcrops. The site has been
excavated near or at groundwater table, allowing for	r the growth of hydrophytic vegetation.
HYDROLOGY	
Wetland Hydrology Indicators:	Secondary Indicators (minimum of two required)
Primary Indicators (minimum of one is required; check all that apply)	Surface Soil Cracks (B6)
Surface Water (A1) Water-Stained Leaves	
X High Water Table (A2) Aquatic Fauna (B13)	Moss Trim Lines (B16)
Saturation (A3) Mart Deposits (B15)	Dry-Season Water Table (C2)
✓ Water Marks (B1) ✓ Hydrogen Sulfide Odor ✓ Sediment Deposits (B2) ✓ Oxidized Rhizospheres	- I - I - I - I - I - I - I - I - I - I
Drift Deposits (B3) Presence of Reduced !	
Algal Mat or Crust (B4) Recent Iron Reduction	
Iron Deposits (B5) Thin Muck Surface (C7) Shailow Aquitard (D3)
Inundation Visible on Aerial Imagery (B7) Other (Explain in Rema	arks) Microtopographic Relief (D4)
Sparsely Vegetated Concave Surface (B8)	FAC-Neutral Test (D5)
Field Observations:	
Surface Water Present? Yes No X Depth (inches):	
Water Table Present? Yes X No Depth (inches):	The state of the s
Saturation Present? Yes No X Depth (inches):	Wetland Hydrology Present? Yes No X
Describe Recorded Data (stream gauge, monitoring well, aerial photos, previ	ous inspections), if available:
Remarks:	
Areas show signs of high water table due to excava	tion of material.

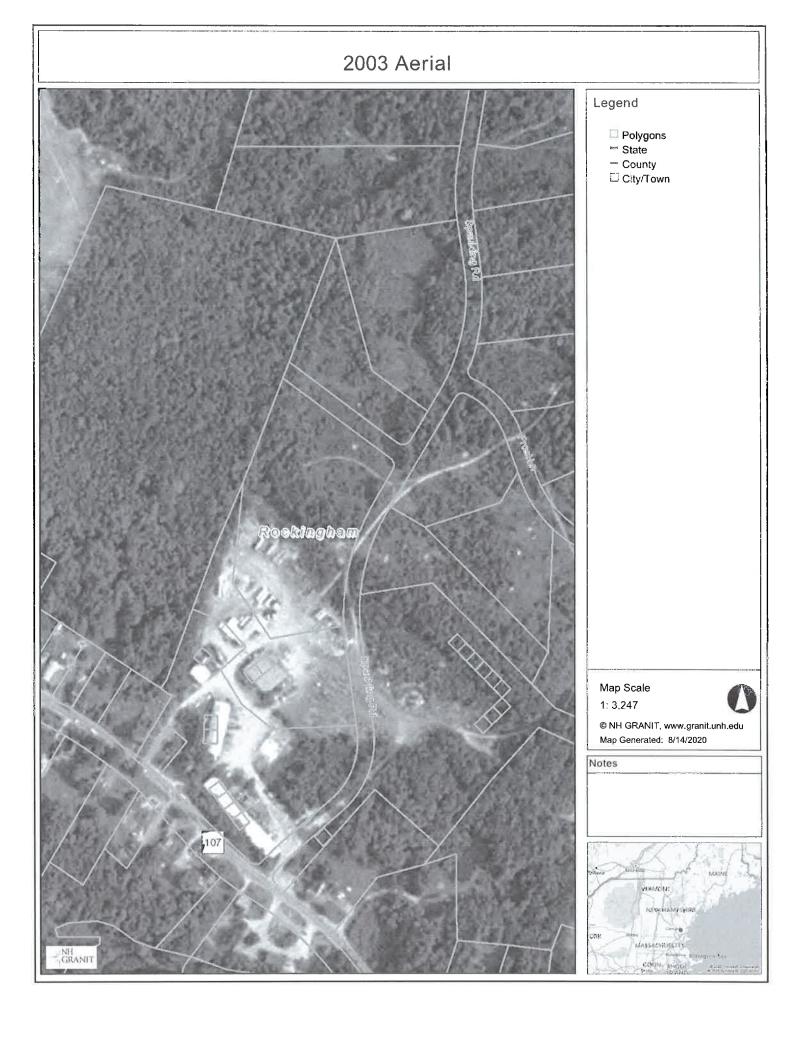
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- 1	м

Tree Stratum (Plot size: 30')	Absolute	Dominant Species?		Dominance Test worksheet:
1. N/A	_/6 Cover	Species:	Status	Number of Dominant Species That Are OBL FACW or FAC: (A)
2				That Are OBL, FACW, or FAC: (A)
3				Total Number of Dominant Species Across All Strata: (B)
4				Percent of Dominant Species That Are OBL, FACW, or FAC: (A/B)
5				
6				Prevalence Index worksheet:
7				Total % Cover of: Multiply by:
15'		= Total Cov	er	OBL species x 1 = _1
Sapling/Shrub Stratum (Plot size: 15')	25	Yes	FACW	FACW species x 2 =
1. Willow (Salix Spp.)				FAC species x 3 = FACU species x 4 =
Quaking aspen (Populus tremuloides)	<u> </u>	No	FACU	UPL species x 5 =
3				Column Totals: (A) (B)
4				
5				Prevalence Index = B/A =
6				Hydrophytic Vegetation Indicators:
7				Rapid Test for Hydrophytic Vegetation
	30	= Total Cov	er	Dominance Test is >50%
Herb Stratum (Plot size: 5'				Prevalence Index is ≤3.0¹
1. Birdsfoot trefoil (Lotus corniculatus)	15	Yes	FACW	Morphological Adaptations ¹ (Provide supporting data in Remarks or on a separate sheet)
2. Tall scouring-rush (Equisetum hyemale)	15	Yes	FACW	Problematic Hydrophytic Vegetation (Explain)
Purple loosestrife (Lythrum salicaria)	10	No	FACW	Troblematic Hydrophytic Vegetation (Explain)
Blunt broom sedge (carex tribuloides)	5	No	FACW	¹ Indicators of hydric soil and wetland hydrology must
5. Bebb willow (Salix bebbiana)	10	No	FACW	be present, unless disturbed or problematic.
5. Debb willow (Salix bebbiaria)		No	FACW	Definitions of Vegetation Strata:
6. Boneset thoroughwort (Eupatorium perfoliatum)	<u> </u>	140	FACVV	Tree – Woody plants 3 in. (7.6 cm) or more in diameter
7				at breast height (DBH), regardless of height.
8				Sapling/shrub – Woody plants less than 3 in. DBH
9				and greater than 3,28 ft (1 m) tall.
10				Herb Ali herbaceous (non-woody) plants, regardless
11				of size, and woody plants less than 3.28 ft tall.
12				Woody vines – All woody vines greater than 3.28 ft in
	60	= Total Cov	er	height.
Woody Vine Stratum (Plot size:)				
1. N/A				
2.				
				I bodon bodin
3				Hydrophytic Vegetation
4				Present? Yes X No
Remarks: (Include photo numbers here or on a separate s		= Total Cov	er	
No trees present. Area is an old sand a	nd grav	el pit wi	th regei	nerating growth.
·				

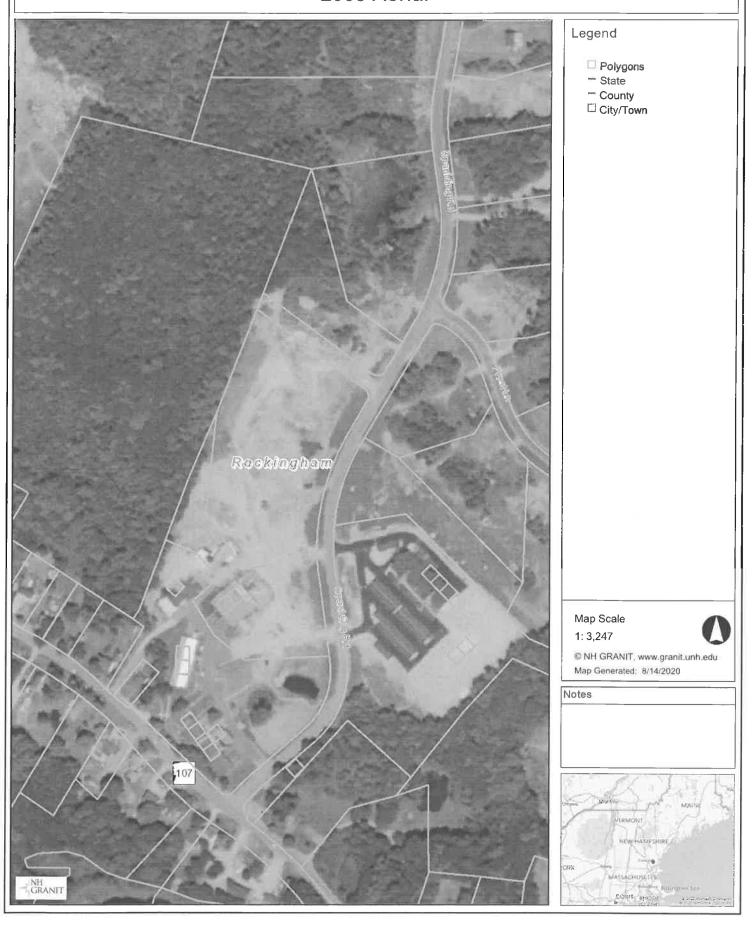
Sampling Point: 1A

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)							
Depth	Matrix			Feature	S		
(inches)	Color (moist)	%	Color (moist)	%	Type ¹	Loc ²	Texture Remarks
0-20	2.5Y 4/4		7.5YR 5/6	5	C	M	Sand
			2.5Y 6/2	5	D	M	
			2.01 0/2			371	
						-	
4							
							· · · · · · · · · · · · · · · · · · ·
-						-	
				-			
W .					-		2
	oncentration, D=Depleti	on, RM=	Reduced Matrix, CS	=Covered	d or Coate	ed Sand G	
Hydric Soil							Indicators for Problematic Hydric Soils ³ :
Histoso!	, ,		Polyvalue Below	/ Surface	(S8) (LR	RR,	2 cm Muck (A10) (LRR K, L, MLRA 149B)
	pipedon (A2)		MLRA 149B)				Coast Prairie Redox (A16) (LRR K, L, R)
Biack Hi		ļ	Thin Dark Surfa	. , .	-		
	n Sulfide (A4)	ļ	Loamy Mucky M			(, L)	Dark Surface (S7) (LRR K, L)
	Layers (A5)	ļ	Loamy Gleyed N		.)		Polyvalue Below Surface (S8) (LRR K, L)
	d Below Dark Surface (A	A11)	Depleted Matrix				Thin Dark Surface (S9) (LRR K, L)
	ark Surface (A12)		Redox Dark Sur				Iron-Manganese Masses (F12) (LRR K, L, R)
	lucky Mineral (S1)		Depleted Dark S		7)		Pledmont Floodplain Soils (F19) (MLRA 149B)
	eleyed Matrix (S4)		Redox Depressi	ons (F8)			Mesic Spodic (TA6) (MLRA 144A, 145, 149B)
	tedox (S5)						Red Parent Material (F21)
	Matrix (S6)						Very Shailow Dark Surface (TF12)
Dark Su	rface (S7) (LRR R, ML f	RA 149B	3)				Other (Explain in Remarks)
3							W = 1.0
	hydrophytic vegetation	and we	tland hydrology mus	be prese	ent, unles	s disturbe	d or problematic.
Restrictive I	_ayer (if observed):						
Type:							
Depth (inc	ches):						Hydric Soil Present? Yes No X
Remarks:							
T.	vpical conditions	s are	not present or	site.	Sand a	and gra	avel have been excavated from the
	te at or near gro		•				
	to at or floar give	, w. i. w. i.	ato, table,				
			182				

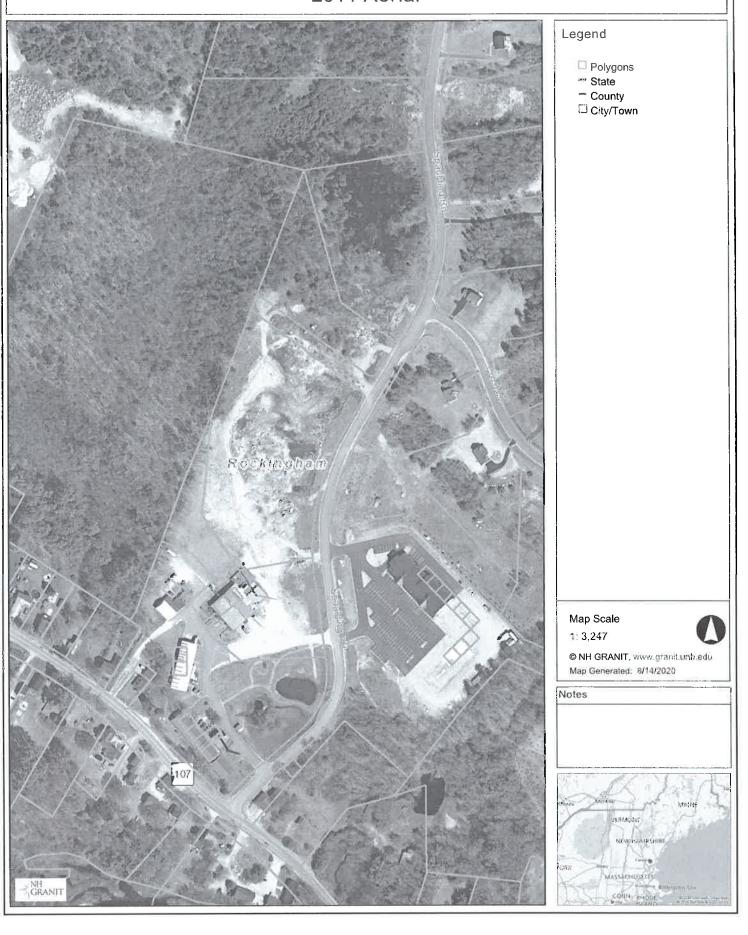


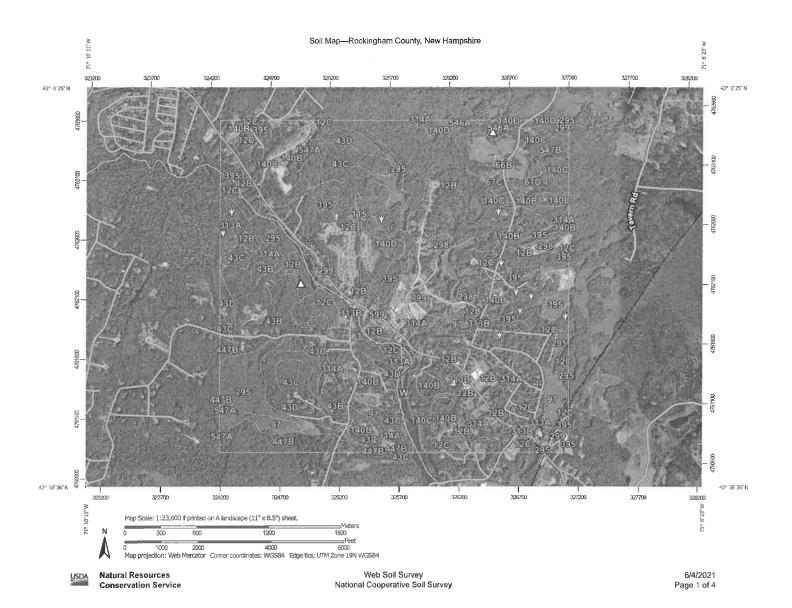


2008 Aerial



2011 Aerial





MAP LEGEND **MAP INFORMATION** The soit surveys that comprise your AOI were mapped at Area of Interest (AOI) Spoil Area 1:24,000. Area of Interest (AOI) Stony Spot â Please rely on the bar scale on each map sheet for map Soils Very Stony Spot 03 Soil Map Unit Polygons Wet Spot 19 Source of Map: Natural Resources Conservation Service Soil Map Unit Lines -Web Soil Survey URL: Δ Other Coordinate System: Web Mercator (EPSG:3857) Soil Map Unit Points ** Special Line Features Special Point Features Maps from the Web Soil Survey are based on the Web Mercator Water Features projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Blowout (0) Streams and Canals Borrow Pit 100 Albers equal-area conic projection, should be used if more Transportation accurate calculations of distance or area are required. Clay Spot 簽 1 1 1 This product is generated from the USDA-NRCS certified data as Closed Depression 0 Interstate Highways proper of the version date(s) listed below. Gravel Pit X US Routes Soil Survey Area: Rockingham County, New Hampshire Gravelly Spot Survey Area Data: Version 22, May 29, 2020 10 Major Reads Landfill Soil map units are labeled (as space allows) for map scales 459 Local Roads Lava Flow A Background Date(s) aerial images were photographed: Mar 30, 2011—Sep Marsh or swamp Aerial Photography ds 200 Mine or Quarry 衆 The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background Miscellaneous Water 0 imagery displayed on these maps. As a result, some minor Perennial Water 0 shifting of map unit boundaries may be evident. Rock Outcrop Saline Spot Severely Eroded Spot et. Sinkhole b Slide or Slip Sodic Spot

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
12B	Hinckley loamy sand, 3 to 8 percent slopes	357.5	17.7%
12C	Hinckley loamy sand, 8 to 15 percent slopes	414.4	20.5%
43B	Canton fine sandy loam, 0 to 8 percent slopes, very stony	92.8	4.6%
43C	Canton fine sandy loam, 8 to 15 percent slopes, very stony	106.0	5.3%
43D	Canton fine sandy loam, 15 to 25 percent slopes, very stony	72.3	3.6%
66B	Paxton fine sandy loam, 3 to 8 percent slopes	14.3	0.7%
67C	Paxton fine sandy loam, 8 to 15 percent slopes, very stony	40.6	2.0%
67D	Paxton fine sandy loam, 15 to 25 percent slopes, very stony	7.8	0.4%
97	Freetown and Natchaug mucky peats, ponded, 0 to 2 percent slopes	52.8	2.6%
115	Scarboro muck, coastal lowland, 0 to 3 percent slopes	3.7	0.2%
140B	Chatfield-Hollis-Canton complex, 0 to 8 percent slopes, rocky	143.3	7.1%
140C	Chatfield-Hollis-Canton complex, 8 to 15 percent slopes, rocky	99.6	4.9%
140D	Chatfield-Hollis-Canton complex, 15 to 35 percent slopes, rocky	26.0	1.3%
295	Freetown mucky peat, 0 to 2 percent slopes	110.4	5.5%
298	Pits, sand and gravel	24.1	1.2%
299	Udorthents, smoothed	17.0	0.8%
313A	Deerfield loamy fine sand, 0 to 3 percent slopes	24.0	1.2%
313B	Deerfield loamy fine sand, 3 to 8 percent slopes	53.7	2.7%
314A	Pipestone sand, 0 to 5 percent slopes	133.2	6.6%

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
395	Swansea mucky peat, 0 to 2 percent slopes	65.6	3.2%
447B	Scituate-Newfields complex, 3 to 8 percent slopes, very stony	56.0	2.8%
546A	Walpole very fine sandy loam, 0 to 5 percent slopes	8.2	0.4%
547A	Walpole very fine sandy loam, 0 to 3 percent slopes, very stony	7.6	0.4%
547B	Waipole very fine sandy loam, 3 to 8 percent slopes, very stony	4.3	0.2%
599	Urban land-Hoosic complex, 3 to 15 percent slopes	11.0	0.5%
799	Urban land-Canton complex, 3 to 15 percent slopes	51.0	2.5%
W	Water	21.8	1.1%
Totals for Area of Interest		2,019,2	100.0%





Aerial Photo of Site

Fremont, NH

1 inch = 275 Feet

CAI Technologies

www.cai-tech.com

October 5, 2021

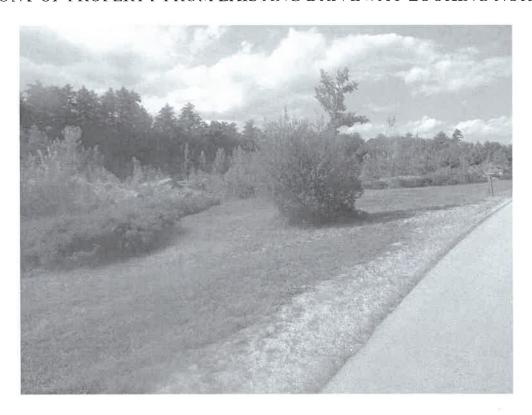
275 551 826



Data shown on this map is provided for planning and informational purposes only. The municipality and CAI Technologies are not responsible for any use for other purposes or misuse or misrepresentation of this map.



FRONT OF PROPERTY FROM EXISTING DRIVEWAY LOOKING NORTH



VIEW FROM SPAULDING RD.

STORMWATER MANAGEMENT OPERATION AND MAINTENANCE MANUAL

Prepared for:

PPM Fremont Holding, LLC Tax Map 3, Lot 37-3 & 37-1 Spalding Road Fremont, NH 03044

> October 6, 2021 JBE Project No. 20631



Inspection and Maintenance of Facilities and Property

A. Maintenance of Common Facilities or Property

The PPM Fremont Holding LLC, future owners and assigns are responsible to perform the
maintenance obligations or hire a Professional Engineer to review the site on an annual basis for
maintenance and certification of the stormwater system. The owner shall keep receipts and records
of all maintenance companies hired throughout the year to submit along with the following form.
The annual report and certification shall be submitted with three copies to the Town Planner by
December 31st of each year.

B. General Inspection and Maintenance Requirements

- a. Permanent stormwater and sediment and erosion control facilities to be maintained on the site include, but are not limited to, the following:
 - a. Swales & Plunge Pools
 - b. Vegetation and landscaping
 - c. Parking lots and roadways
 - d. Riprap inlet and outlet protection aprons
- b. Maintenance of permanent measures shall follow the following schedule:
 - a. Normal winter roadway and parking lot maintenance including plowing and snow removal.
 - b. Road and parking lot sweeping at the end of every winter, preferably at the start of the spring rain season.
 - c. **Inspection** of culvert inlets and outlets at least **once per month** during the rainy season (March to November). Any debris is to be removed and disposed of properly by residents or contractor.
 - d. **Annual inspection** of the site for erosion, destabilization, settling, and sloughing. Any needed repairs are to be conducted immediately.
 - e. Annual inspection of site's vegetation and landscaping. Any areas that are bare shall be reseeded and mulched with hay or, if the case is extreme, loamed and seeded or sodded to ensure adequate vegetative cover. Landscape specimens shall be replaced in kind, if they are found to be dead or dying.
 - f. Permanent stone check dams should be **inspected annually** in order to ensure that they are in good condition. Any sediment accumulated behind them shall be removed if it is deeper than six inches.
 - g. Rock riprap should be **inspected annually** in order to ensure that it has not been displaced, undermined, or otherwise damaged. Displaced rock should be replaced, or additional rock added in order to maintain the structure(s) in their undamaged state. Woody vegetation should not be allowed to become established in riprap areas, and/or any debris removed from the void spaces between the rocks. If the riprap is adjacent to a stream or other waterbody, the water should be kept clear of obstructions, debris, and sediment deposits.



Winter maintenance:

- Sanding for winter traction is prohibited. Deicing is permitted (NaCl, MgCl₂, or equivalent).
 Reduced salt application is possible and can be a cost savings for winter maintenance.
 Nontoxic, organic deicers, applied either as blended, magnesium chloride-based liquid products or as pretreated salt, are preferable.
- Plowing is allowed, blade should be set approximately 1" above road surface. Ice and light snow accumulation are generally not as problematic as for standard asphalt. Snow will accumulate during heavier storms and should be plowed. (more than usual, about an inch).

Routine maintenance:

- Asphalt seal coating is absolutely forbidden. Surface seal coating is not reversible.
- The pavement surface should be vacuumed 2 or 3 times per year, and at any additional times sediment is spilled, eroded, or tracked onto the surface.
- Planted areas adjacent to pervious pavement should be well maintained to prevent soil washout onto the pavement. If any bare spots or eroded areas are observed within the planted areas, they should be replanted and/or stabilized at once.
- Immediately clean any soil deposited on pavement. Superficial dirt does not necessarily clog the pavement voids. However, dirt that is ground in repeatedly by tires can lead to clogging. Therefore, trucks or other heavy vehicles should be prevented from tracking or spilling dirt onto the pavement.
- Do not allow construction staging, soil/mulch storage, etc. on unprotected pavement surface. Contractor to laydown tarps, plywood or removable item and take care not to track material onto unprotected pavement.
- Repairs: potholes of less than 50 square feet can be patched by any means suitable with standard pavement or a pervious mix is preferred. For areas greater than 50 sq. ft. is in need of repair, approval of patch type should be sought from a qualified engineer. Any required repair of drainage structures should be done promptly to ensure continued proper functioning of the system.
- Written and verbal communication to the porous pavement's future owner should make clear the pavement's special purpose and special maintenance requirements such as those listed here.
- A permanent sign should be added at the entrance and end of the porous asphalt area to inform residents and maintenance staff of the special nature and purpose of the pavement, and its special maintenance requirements.

Signage should read as follows:

POROUS ASPHALT PAVEMENT FOR STORM WATER MANAGEMENT

MAINTENANCE REQUIREMENTS:

PLOW WITH SLIGHTLY RAISED BLADE ONLY
SANDING OF SURFACE PROHIBITED
DEICING PERMITTED (NaC1, MgC12 OR EQUIVALENT)
SEAL-COATING PROHIBITED
CLEANING BY PRESSURIZED AIR OR WATER PROHIBITED
DRY VACUUM SEMIANNUALLY



h. Treatment Swales:

Inspect annually for erosion, sediment accumulation, vegetation loss, and presence of invasive species. Perform periodic mowing; frequency depends on location and type of grass. Do not cut shorter than Water Quality Flow depth (maximum 4-inches). Remove debris and accumulated sediment, based on inspection. Repair eroded areas, remove invasive species and dead vegetation, and reseed with applicable grass mix as warranted by inspection.

i. Vegetated Buffers:

Inspect buffer at least annually for signs of erosion, sediment buildup, or vegetation loss. If a meadow buffer, provide periodic mowing as needed to maintain a healthy stand of herbaceous vegetation. If a forested buffer, then the buffer should be maintained in an undisturbed condition, unless erosion occurs. If erosion of the buffer (forested or meadow) occurs, eroded areas should be repaired and replanted with vegetation similar to the remaining buffer. Corrective action should include eliminating the source of the erosion problem, and may require retrofit with a level spreader. Remove debris and accumulated sediment, based on inspection.

n. Detention ponds should require little maintenance, but should be inspected frequently during the first year of operation, and annually thereafter. Every five years, the services of a professional engineer should be retained to perform a thorough inspection of all the aspects of the pond and its infrastructure. Any debris and sediment accumulations should be removed from the outlet structure(s) and emergency spillway(s) and disposed of properly. Inspect outlet structure for deterioration and or elogging. Detention pond berms should be mowed at least once annually so as to prevent the establishment of woody vegetation – trees should never be allowed to grow on a detention pond berm, as they may destabilize the structure and increase the potential for failure. Areas showing signs of erosion or thin or dying vegetation should be repaired immediately by whatever means necessary, with the exception of fertilizer. Rodent burrows are to be repaired immediately and the suspect animals apprehended with non-lethal traps if the problem persists.

See attached sample forms as a guideline.

Any inquiries in regards to the design, function, and/or maintenance of any one of the above mentioned facilities or tasks shall be directed to the project engineer:

Jones & Beach Engineers, Inc. 85 Pertsmouth Avenue P.O. Box 219 Stratham, NH 03885

T#: (603) 772-4746 F#: (603) 772-0227



Commitment to maintenance requirements

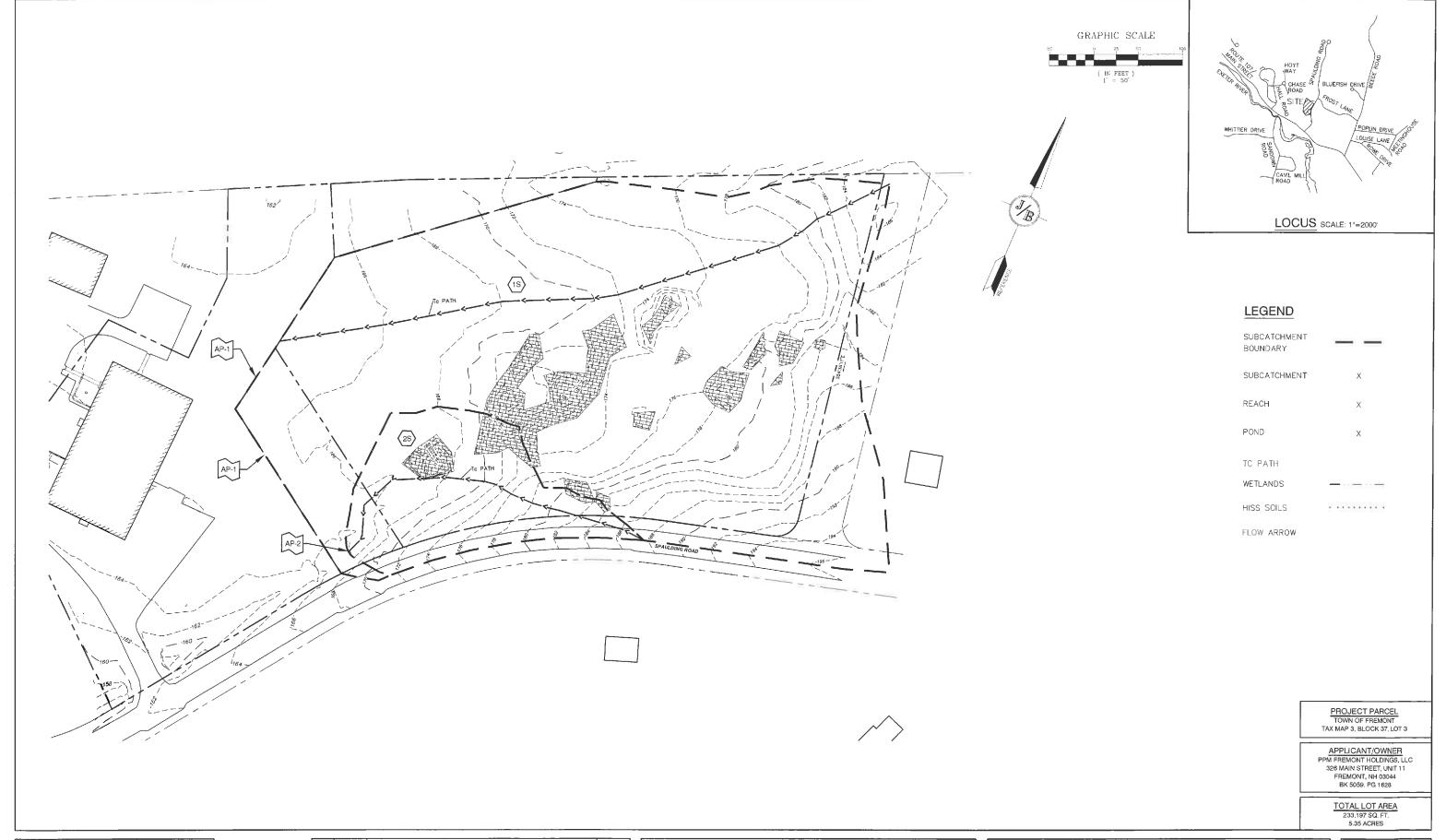
I agree to complete and/or observe all of the reschedules as outlined above.	equired maintenance practices and their respective
Signature	
Print Name	-
Title	
Date	

Annual Operations and Maintenance Report

The PPM Fremont Holding, LLC, future owners and assigns are responsible to perform the maintenance obligations or hire a Professional Engineer to review the site on an annual basis for maintenance and certification of the stormwater system. The owner shall keep receipts and records of all maintenance companies hired throughout the year to submit along with the following form. The annual report and certification shall be submitted with three copies to the Town Planner by December 31st of each year.

Construction Activity	Date of Inspection	Who Inspected	Findings of Inspector
Catch basins and drain manholes			
Culverts			
Swales & Plunge Pools			
Vegetation and landscaping			
Parking lots and roadways			

Rain Gardens (Bioretention systems)		
Porous Pavement		
Stormtech		
Underground		
Detention System		
Eco-Paver Areas		
Plunge Pools		
Roof Drains		



Design: BWG		Date: 10/6/2021
	Scale: AS SHOWN	
Drawing Name:	20631-WATERSHED	.dwg
THIS PLAN SHAL	NOT BE MODIFIED WIT	HOUT WRITTEN
PERMISSION FRO	M JONES & BEACH EN	GINEERS, INC. (JBE).
ANY ALTERATION	IS, AUTHORIZED OR OT	HERWISE, SHALL BE
AT THE USER'S S	OLE RISK AND WITHOU	T LIABILITY TO JBE.

0	10/6/2021	ISSUED FOR REVIEW	BGW
REV.	DATE	REVISION	BY

Jones & Beach Engineers, Inc.

B5 Portsmouth Ave. PO Box 219
Stratham, NH 03885

Designed and Produced in NH

Beach Engineers, Inc.

B03-772-4746
FAX: 603-772-0227
E-MAIL: JBE@JONESANDBEACH.COM

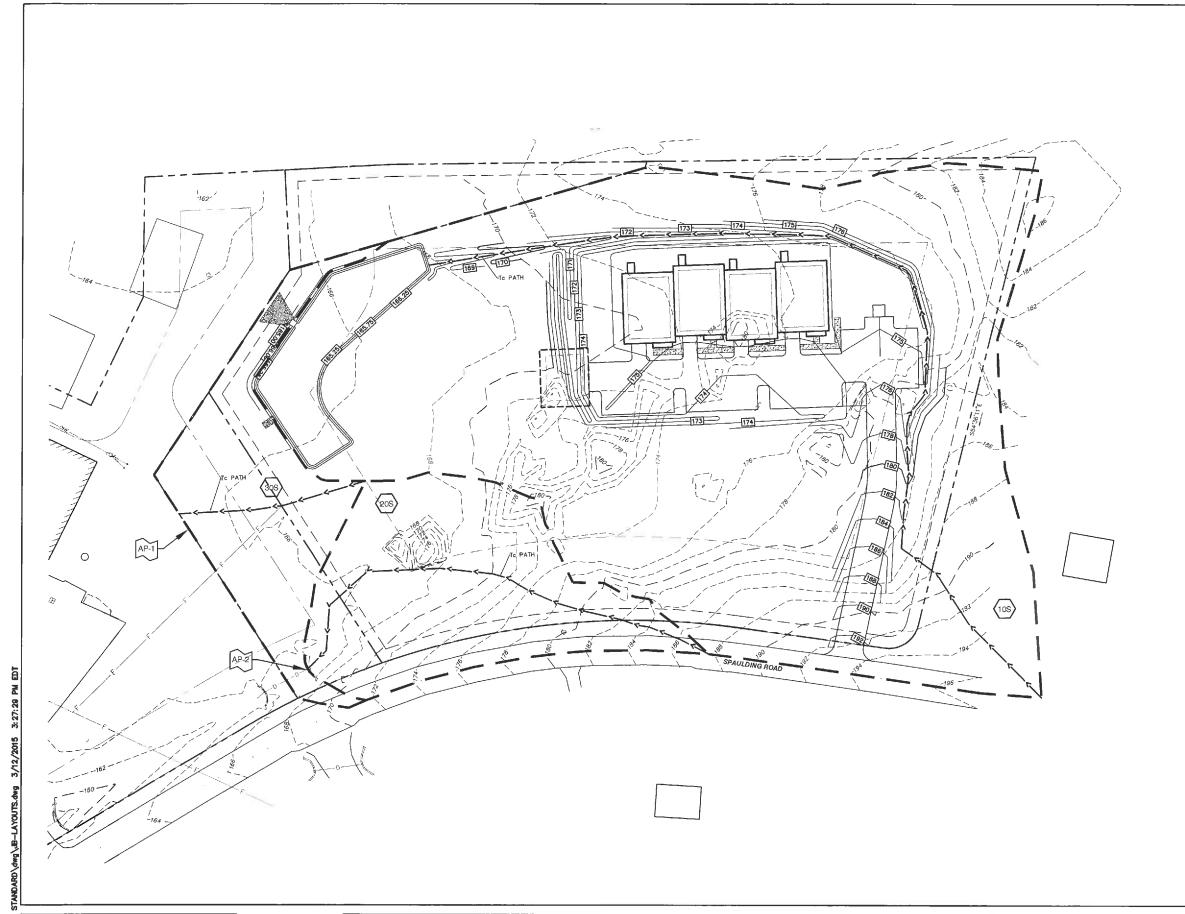
Plan Name:	EXISTING WATERSHED PLAN
Project:	PPM TOWNHOMES SPAULDING ROAD, FREMONT, NH
Owner of Record:	PPM FREMONT HOLDING, LLC 326 MAIN STREET, FREMONT, NH 03044

DRAWING No.

W1

SHEET 1 OF 2

JBE PROJECT NO. 20631







LEGEND

SUBCATCHMENT
BOUNDARY

SUBCATCHMENT

X

REACH

Y

TO PATH

WETLANDS

HISS SOILS

FLOW ARROW

PROJECT PARCEL
TOWN OF FREMONT
TAX MAP 3, BLOCK 37, LOT 3

APPLICANT/OWNER
PPM FREMONT HOLDINGS, LLC
326 MAIN STREET, UNIT 11
FREMONT, NH 03044
BK 5059, PG 1628

TOTAL LOT AREA 233,197 SQ. FT. 5.35 ACRES

Design: BWG Draft: DFP Date: 10/6/2021
Checked: BWG Scale: AS SHOWN Project No.:20831
Drawing Name: 20831-WATERSHED.dwg
THIS PLAN SHALL NOT BE MODIFIED WITHOUT WRITTEN
PERMISSION FROM JONES & BEACH ENGINEERS, INC. (JBE).
ANY ALTERATIONS. AUTHORIZED OR OTHERWISE, SHALL BE
AT THE USER'S SOLE RISK AND WITHOUT UABILITY TO JBE.

			L
0	10/6/2021	ISSUED FOR REVIEW	BGV
REV.	DATE	REVISION	BY

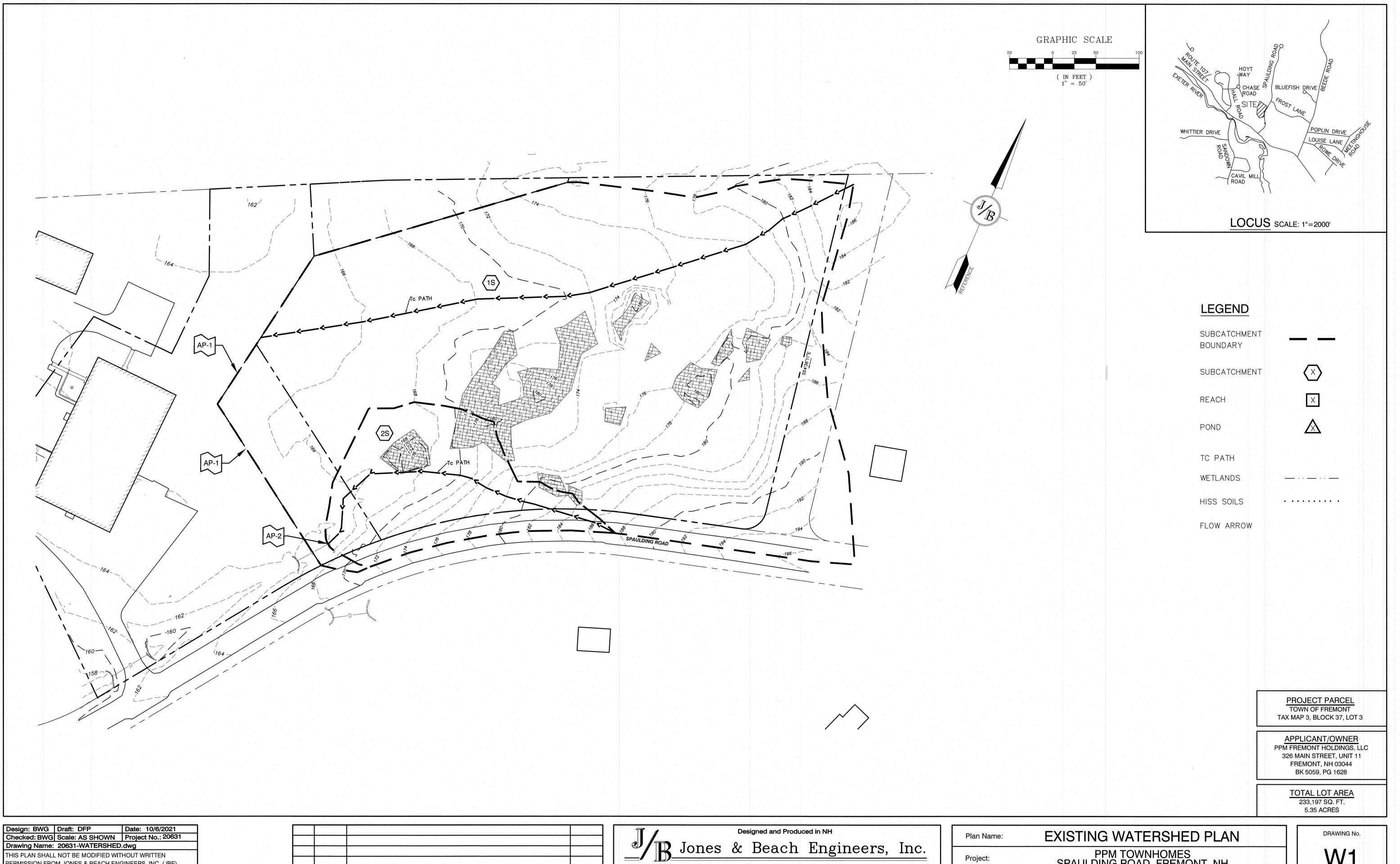
Jones & Beach Engineers, Inc.

B5 Portsmouth Ave. PO Box 219 Stratham, NH 03885 Civil Engineering Services FAX: 603-772-0227 E-MAIL: JBE@JONESANDBEACH.COM

Plan Name:	PROPOSED WATERSHED PLAN
Project:	PPM TOWNHOMES SPAULDING ROAD, FREMONT, NH
Owner of Record:	PPM FREMONT HOLDING, LLC 326 MAIN STREET, FREMONT, NH 03044

DRAWING No.

W2
SHEET 2 OF 2
JBE PROJECT NO. 20631



Drawing Name: 20631-WATERSHED.dwg PERMISSION FROM JONES & BEACH ENGINEERS, INC. (JBE). ANY ALTERATIONS, AUTHORIZED OR OTHERWISE, SHALL BE AT THE USER'S SOLE RISK AND WITHOUT LIABILITY TO JBE.

			(4) a
3.4			
0	10/6/2021	ISSUED FOR REVIEW	BGW
REV.	DATE	REVISION	BY

Services

FAX: 603-772-4746

FAX: 603-772-0227

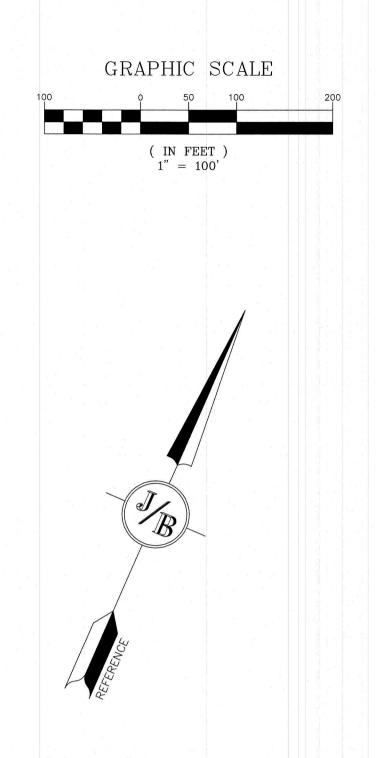
E-MAIL: JBE@JONESANDBEACH.COM 85 Portsmouth Ave. Civil Engineering Services
PO Box 219
Stratham, NH 03885

Civil Engineering Services
E-MAIL: JBE@

Plan Name:	EXISTING WATERSHED PLAN	
Project:	PPM TOWNHOMES SPAULDING ROAD, FREMONT, NH	
Owner of Record:	PPM FREMONT HOLDING, LLC 326 MAIN STREET, FREMONT, NH 03044	

SHEET 1 OF 2 JBE PROJECT NO.**20631**





LEGEND

SUBCATCHMENT BOUNDARY

SUBCATCHMENT

REACH

POND

TC PATH

WETLANDS

HISS SOILS

FLOW ARROW

PROJECT PARCEL
TOWN OF FREMONT
TAX MAP 3, BLOCK 37, LOT 3

->-->--

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APPLICANT/OWNER

PPM FREMONT HOLDINGS, LLC

326 MAIN STREET, UNIT 11

FREMONT, NH 03044

TOTAL LOT AREA 233,197 SQ. FT. 5.35 ACRES

BK 5059, PG 1628

Checked: BWG Scale: AS SHOWN Project No.:20631
Drawing Name: 20631-WATERSHED.dwg

THIS PLAN SHALL NOT BE MODIFIED WITHOUT WRITTEN
PERMISSION FROM JONES & BEACH ENGINEERS, INC. (JBE).
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AT THE USER'S SOLE RISK AND WITHOUT LIABILITY TO JBE.

0	10/6/2021	ISSUED FOR REVIEW	BGW
REV.	DATE	REVISION	BY

Book and Produced in NH

Book Beach Engineers, Inc.

Besigned and Produced in NH

Beach Engineers, Inc.

Besigned and Produced in NH

Beach Engineers, Inc.

Besigned and Produced in NH

Besi

Plan Name:	PROPOSED WATERSHED PLAN
Project:	PPM TOWNHOMES SPAULDING ROAD, FREMONT, NH
Owner of Record:	PPM FREMONT HOLDING, LLC 326 MAIN STREET, FREMONT, NH 03044

DRAWING No.

W2

SHEET 2 OF 2

JBE PROJECT NO. 20631